

CITY OF BARDSTOWN
D.B. 233, PG. 200
PARCEL #45000-00-169.08

N: 2186215.90
E: 1723064.98

KARINA L. DOWNS
LOTS 19-22
D.B. 512, PG. 434 & 437
PARCEL #45NNE-03-001

WITHROW COURT SUBDIVISION
P.B. 1, PG. 08

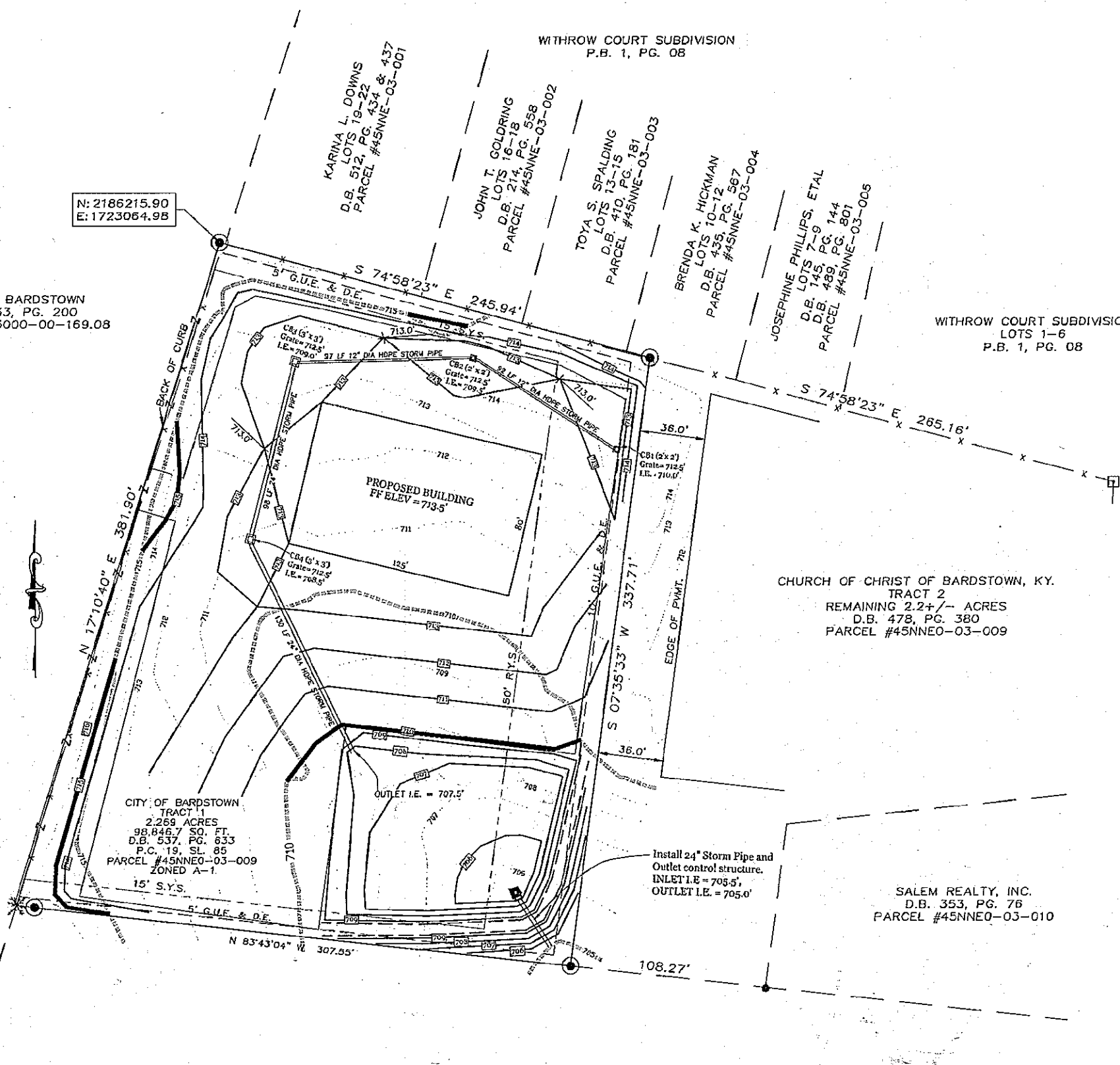
JOHN T. GOLDRING
LOTS 16-18
D.B. 214, PG. 558
PARCEL #45NNE-03-002

TOYA S. SPALDING
LOTS 13-15
D.B. 410, PG. 181
PARCEL #45NNE-03-003

BRENDA K. HICKMAN
LOTS 10-12
D.B. 435, PG. 567
PARCEL #45NNE-03-004

JOSEPHINE PHILLIPS, ETAL
LOTS 7-9, 144
D.B. 480, PG. 801
PARCEL #45NNE-03-005

WITHROW COURT SUBDIVISION
LOTS 1-6
P.B. 1, PG. 08



CHURCH OF CHRIST OF BARDSTOWN, KY.
TRACT 2
REMAINING 2.2+/- ACRES
D.B. 478, PG. 380
PARCEL #45NNE0-03-009

CITY OF BARDSTOWN
TRACT 1
2.269 ACRES
98,846.7 SQ. FT.
D.B. 537, PG. 633
P.C. 19, SL. 85
PARCEL #45NNE0-03-009
ZONED A-1
15' S.Y.S.

Install 24" Storm Pipe and
Outlet control structure.
INLET I.E. = 705.5',
OUTLET I.E. = 705.0'

SALEM REALTY, INC.
D.B. 353, PG. 76
PARCEL #45NNE0-03-010

COMMONWEALTH OF KENTUCKY
DEPARTMENT OF MILITARY AFFAIRS
D.B. 254, PG. 354
PARCEL #45NNE0-03-011

N: 2185688.90
E: 1722901.94

S 17°13'11" W
169.75'

N 83°43'04" W
307.55'

108.27'

CITY OF BARDSTOWN
D.B. 233, PG. 200
PARCEL #45000-00-169.08

N: 2186215.90
E: 1723064.98

KARINA L. DOWNS
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PARCEL #45NNE-03-001

JOHN T. GOLDRING
D.B. 214, PG. 558
PARCEL #45NNE-03-002

TOYA S. SPALDING
D.B. 410, PG. 181
PARCEL #45NNE-03-003

BRENDA K. HICKMAN
D.B. 435, PG. 567
PARCEL #45NNE-03-004

JOSEPHINE PHILLIPS, ETAL
D.B. 145, PG. 144
D.B. 489, PG. 801
PARCEL #45NNE-03-005

WITHROW COURT SUBDIVISION
LOTS 1-6
P.B. 1, PG. 08

CHURCH OF CHRIST OF BARDSTOWN, KY.
TRACT 2
REMAINING 2.2+/- ACRES
D.B. 478, PG. 380
PARCEL #45NNE0-03-009

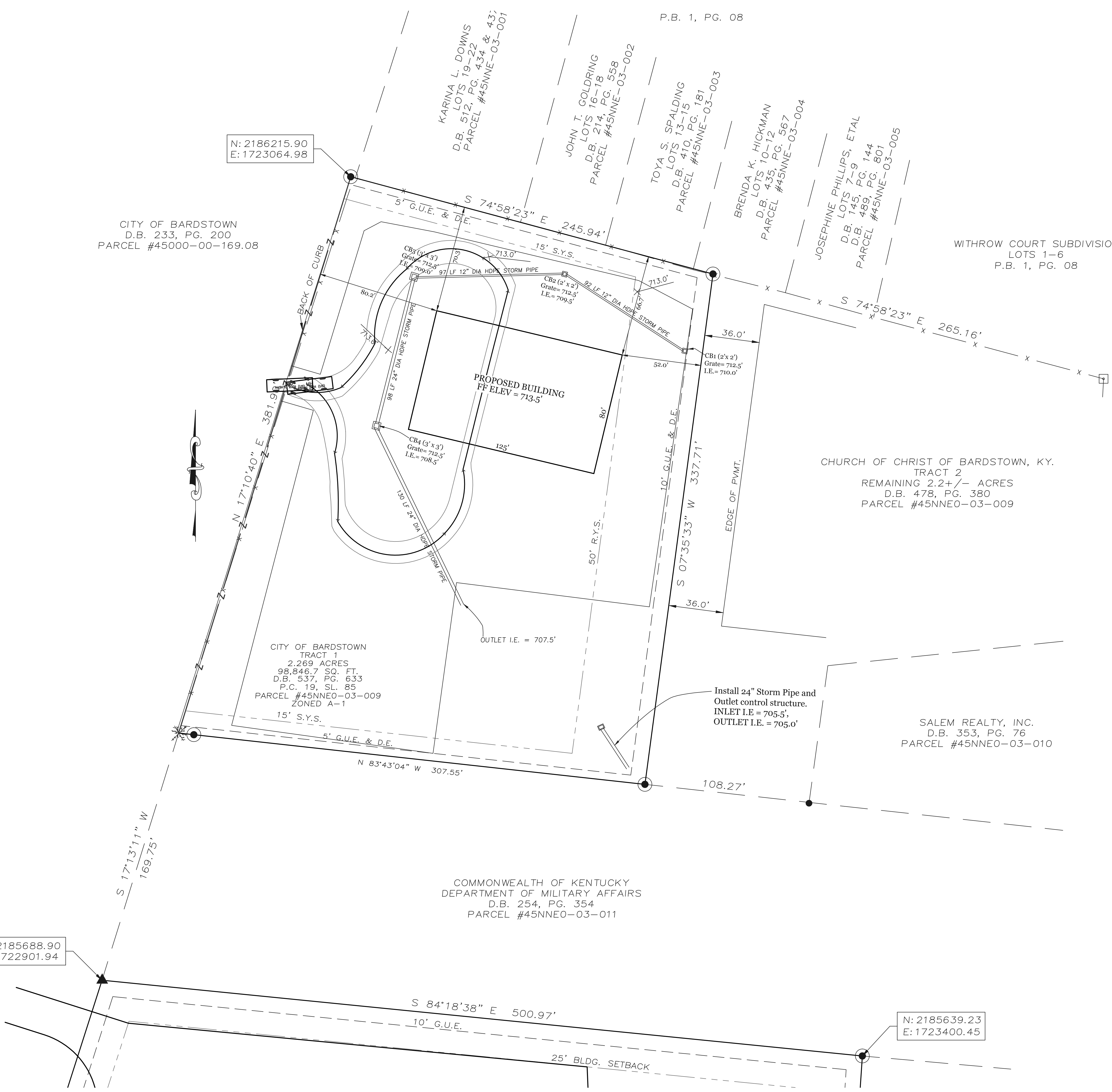
SALEM REALTY, INC.
D.B. 353, PG. 76
PARCEL #45NNE0-03-010

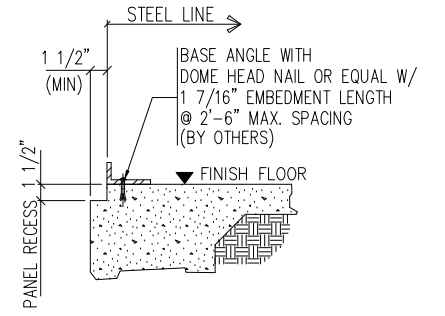
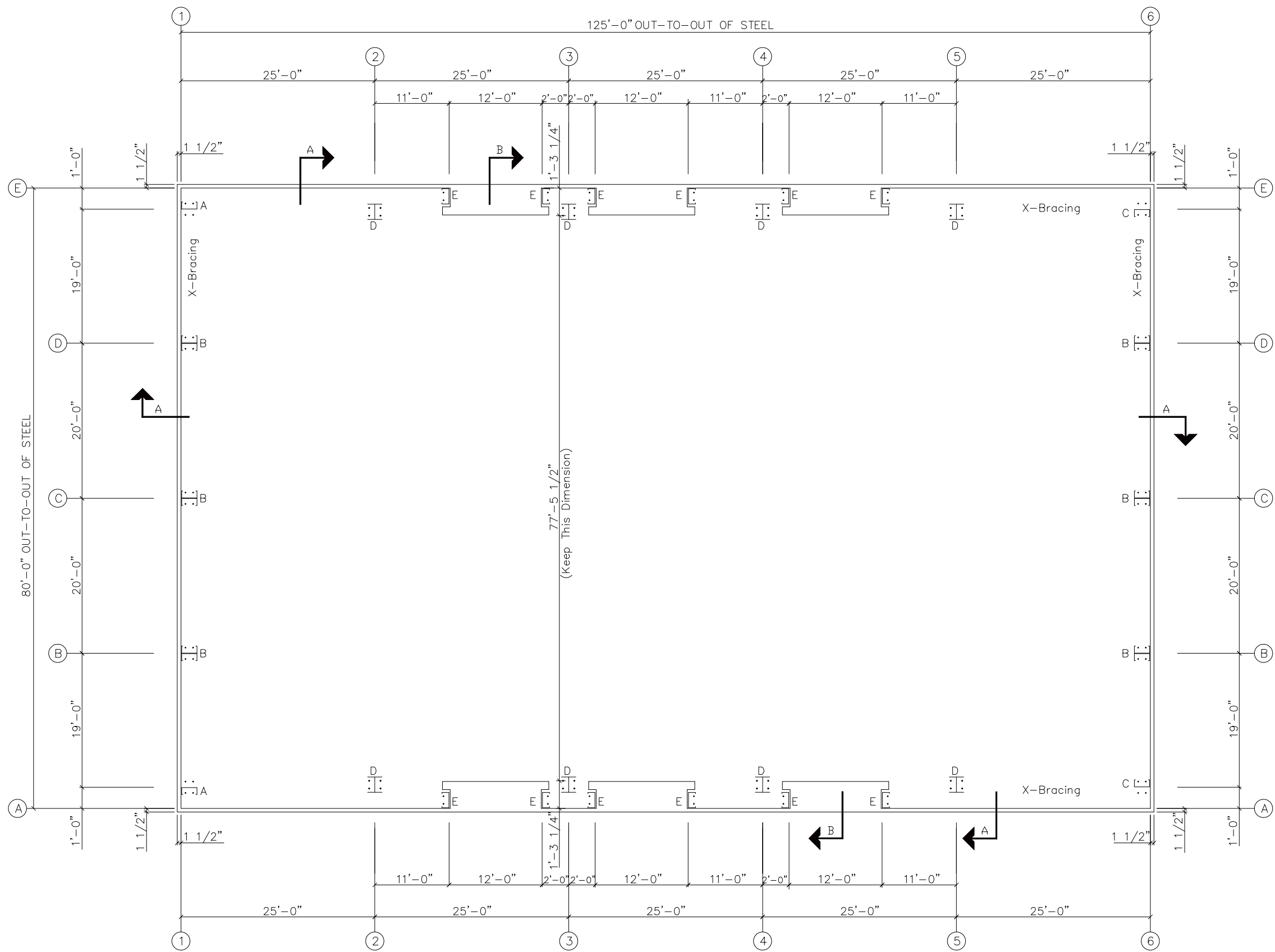
COMMONWEALTH OF KENTUCKY
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D.B. 254, PG. 354
PARCEL #45NNE0-03-011

CITY OF BARDSTOWN
TRACT 1
2.269 ACRES
98,846.7 SQ. FT.
D.B. 537, PG. 633
P.C. 19, SL. 85
PARCEL #45NNE0-03-009
ZONED A-1

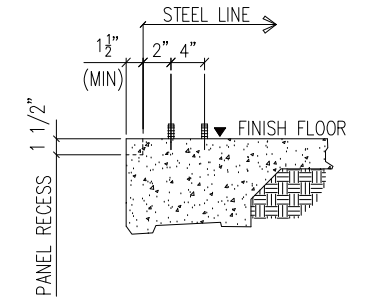
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E: 1722901.94

N: 2185639.23
E: 1723400.45

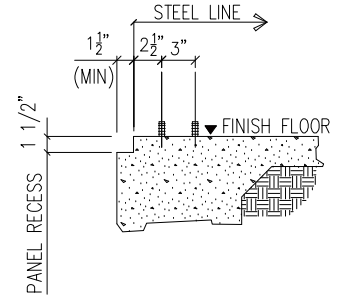




A SECTION



B SECTION



FIELD LOCATE WALKDOOR

COLUMN LAYOUT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

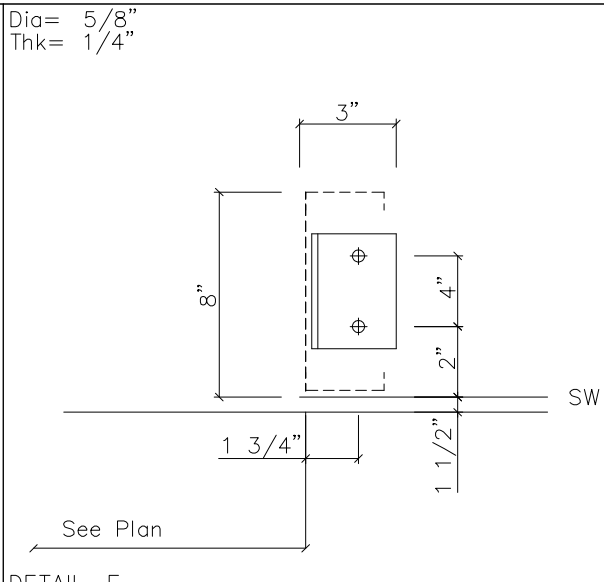
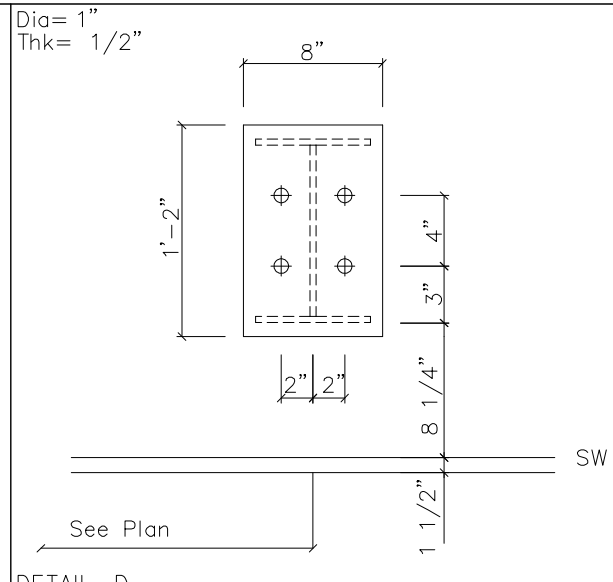
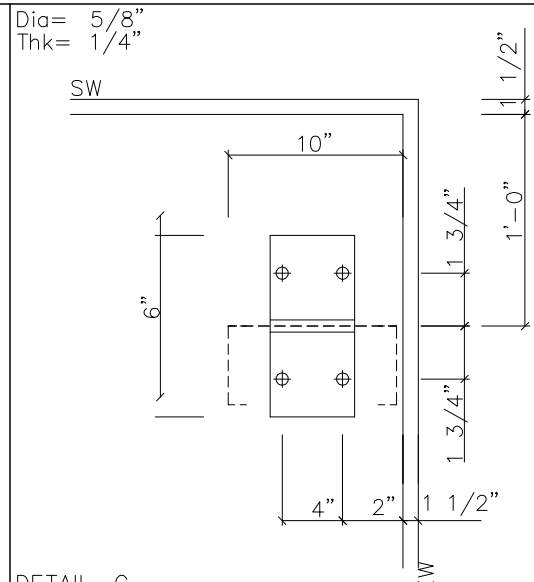
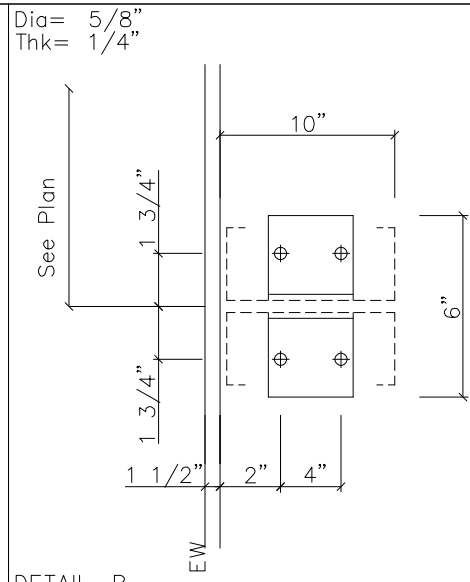
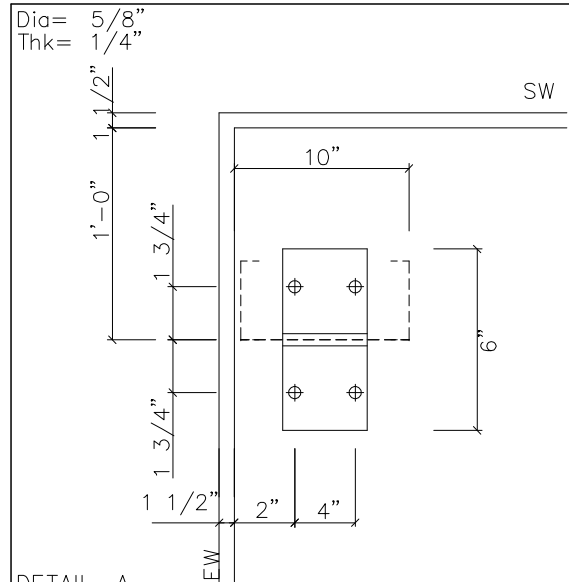
SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT DURO BEAM ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY D.B.S.B. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN D.B.S.B. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

FOR CONSTRUCTION

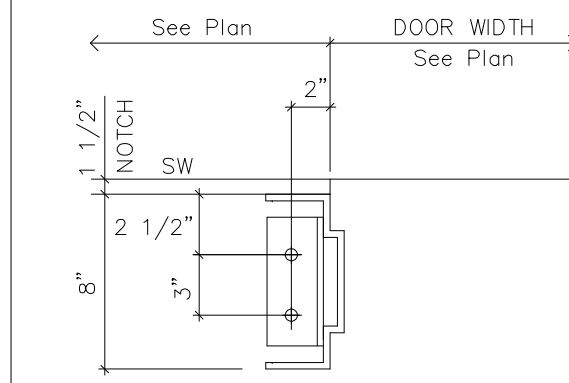
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	PERMIT/CONSTRUCTION	04/13/21	EPN	FLT	LCB



DESCRIPTION	COLUMN LAYOUT PLAN
CUSTOMER	City of Bardstovnn
END USER	City of Bardstovnn
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstovnn, KY 40004
SCALE NO.	70112
SCALE	152536
SCALE	N.T.S.
DWG. NO.	F001
ISSUE	0



Dia= 1/2" Expansion Bolts - For Field Located (BY OTHERS)



FOR CONSTRUCTION

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GENERAL NOTES:
 ① THE ANCHOR BOLT DETAILS SHOWN ON THIS DRAWING LOCATE THE ANCHOR BOLTS IN REFERENCE TO BOTH THE BUILDING STEEL LINE AND THE OUTSIDE OF RIGID'S SUGGESTED PANEL RECESS OF 1-1/2".
 ② THE ANCHOR BOLT SETTING PLAN LOCATES ANCHOR BOLTS IN REFERENCE TO THE OUTSIDE OF THE PANEL RECESS SHOWN. IF THE ACTUAL PANEL RECESS IS DIFFERENT FROM WHAT IS SHOWN ON THE ANCHOR BOLT SETTING PLAN, THEN ALL REFERENCE DIMENSIONS FROM THE OUTSIDE OF THE PANEL RECESS MUST BE DETERMINED BY THE CUSTOMER.
 ③ BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)

NOTE:
 ONLY ANCHOR BOLTS SETTING PLAN ISSUED & STAMPED "FOR CONSTRUCTION" SHALL BE USED IN SETTING ANCHOR BOLTS. "RIGID GLOBAL BUILDINGS" SHALL NOT BE RESPONSIBLE FOR ERROR OR DISCREPANCY IF THE DRAWING USED IS NOT VALID FOR CONSTRUCTION.

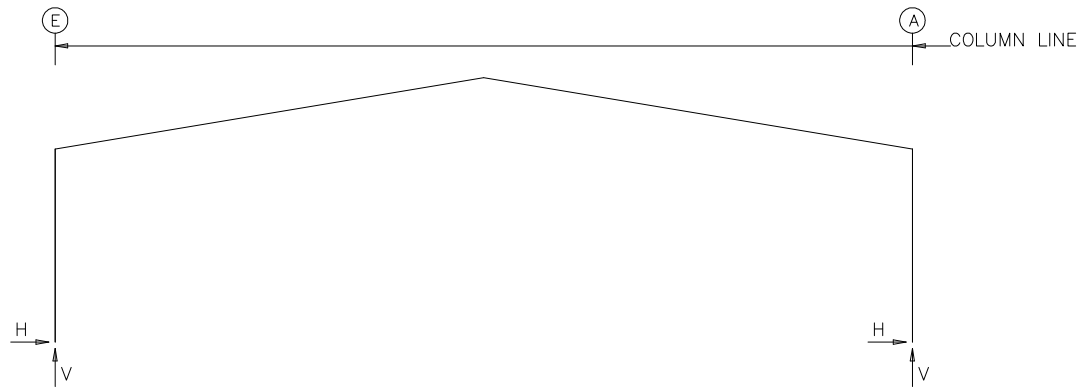
QTY.	SYMBOL	DIA.	PROJ.	ANCHOR BOLT DETAIL
0	+	1/2"	1"	ANCHOR BOLT PROJECTION "PROJ." IS MEASURED FROM BOTTOM OF BASE PLATE
64	+	5/8"	2"	DETAIL OF ANCHOR BOLT AS PER THE SUPPLIER
0	+	3/4"	2 1/2"	
0	+	7/8"	2 3/4"	
36	+	1"	3"	LENGHT OF "PROJ." SHOWN IS FOR ONE NUT + ONE WASHER
0	+	1 1/8"	3 1/2"	NUTS & WASHERS BY SUPPLIER
0	+	1 1/2"	3 1/2"	ANCHOR BOLTS NOT BY RIGID GLOBAL BUILDINGS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	PERMIT/CONSTRUCTION	04/13/21	EPN	FLT	LCB



DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	City of Bardstown
END USER	City of Bardstown
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstown, KY 40004
SALES NO.	70112
JOB NO.	152536
SCALE	N.T.S.
DWG. NO.	F002
ISSUE	0

FRAME LINES: 2 3 4 5



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_Left1---		---Wind_Right1---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	E	2.2	3.5	2.3	3.0	8.9	12.0	14.8	20.0	-16.7	-21.0	-8.1	-15.6
2*	A	-2.2	3.5	-2.3	3.0	-8.9	12.0	-14.8	20.0	8.1	-15.6	16.7	-21.0
Frame Line	Column Line	---Wind_Left2---		---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	E	-12.7	-11.9	-4.1	-6.5	-8.3	-25.0	-9.7	-21.5	-0.5	-0.2	0.5	0.2
2*	A	4.1	-6.5	12.7	-11.9	9.7	-21.5	8.3	-25.0	-0.5	0.2	0.5	-0.2
Frame Line	Column Line	---Seismic_Long---		---MIN_SNOW---		F1UNB_SL_L---		F1UNB_SL_R---					
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert				
2*	E	0.0	-1.8	14.8	20.0	9.2	14.0	9.1	8.3				
2*	A	0.0	-1.8	-14.8	20.0	-9.1	8.3	-9.2	14.0				
Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_Left1---		---Wind_Right1---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
3	E	2.2	3.6	2.3	3.0	8.9	12.0	14.9	20.0	-16.7	-21.0	-8.1	-15.6
3	A	-2.2	3.6	-2.3	3.0	-8.9	12.0	-14.9	20.0	8.1	-15.6	16.7	-21.0
Frame Line	Column Line	---Wind_Left2---		---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
3	E	-12.7	-11.9	-4.1	-6.5	-8.3	-19.7	-9.7	-16.3	-0.5	-0.2	0.5	0.2
3	A	4.1	-6.5	12.7	-11.9	9.7	-16.3	8.3	-19.7	-0.5	0.2	0.5	-0.2
Frame Line	Column Line	---MIN_SNOW---		F2UNB_SL_L---		F2UNB_SL_R---							
		Horiz	Vert	Horiz	Vert	Horiz	Vert						
3	E	14.9	20.0	9.2	14.0	9.2	8.3						
3	A	-14.9	20.0	-9.2	8.3	-9.2	14.0						
2*	Frame lines:	2 4 5											

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	E	6	19.3	26.5	2	-8.7	-10.5	4	1.000	8.000	14.00	0.500	0.0
		1	19.3	26.5	4	-3.7	-12.9						
2*	A	3	8.7	-10.5	6	-19.3	26.5	4	1.000	8.000	14.00	0.500	0.0
		1	-19.3	26.5	5	3.7	-12.9						
2*	Frame lines:	2 4 5											

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
3	E	6	19.4	26.6	2	-8.7	-10.5	4	1.000	8.000	14.00	0.500	0.0
		1	19.4	26.6									
3	A	3	8.7	-10.5	6	-19.4	26.6	4	1.000	8.000	14.00	0.500	0.0
		1	-19.4	26.6	3	8.7	-10.5						

Zone	Width (ft)	Length (ft)	Components & Cladding (Factored)			
			Pressure(psf) Member	Panel	Suction(psf) Member	Panel
6	7.20	7.20	10.08	10.85	-14.89	-16.41
			10.08	10.85	-21.00	-28.39
			10.08	10.85	-21.00	-28.39
			10.08	10.85	-33.06	-42.17
			13.98	16.44	-15.36	-17.76
			13.98	16.44	-16.44	-21.84

Design Calculation Wind

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	ANCHOR BOLT REACTIONS
0	PERMIT/CONSTRUCTION	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstovnn
						END USER	City of Bardstovnn
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstovnn, KY 40004
						SALES NO.	70112
						JOB NO.	152536
						SCALE	N.T.S.
						DWG. NO.	F003
						ISSUE	0



ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	E	0.4	0.3	2.0	2.1	2.5	-5.5	0.0	0.7	2.5	-4.3	0.0	1.8	-1.9
1	D	1.0	0.9	5.4	5.7	0.0	-6.7	2.5	-7.7	0.0	-4.0	2.5	-5.1	-4.3
1	C	1.0	0.7	4.5	4.8	0.0	-4.7	0.0	-5.5	0.0	-3.1	0.0	-3.9	-5.1
1	B	1.0	0.9	5.4	5.7	0.0	-5.1	0.0	-8.8	0.0	-2.5	0.0	-6.2	-4.3
1	A	0.4	0.3	2.0	2.1	0.0	-2.3	0.0	-2.9	0.0	-1.1	0.0	-1.7	-1.9

Frm Line	Col Line	Wind Suct Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	-MIN_SNOW-- Horz	-MIN_SNOW-- Vert	E1UNB_SL_L-- Horz	E1UNB_SL_L-- Vert
1	E	2.2	0.0	-1.8	1.1	-3.0	0.6	-0.6	0.0	0.8	0.0	2.1	0.0	1.3	0.0
1	D	4.8	1.1	-10.0	0.0	-3.9	0.0	0.6	0.6	-0.7	0.0	5.7	0.0	4.8	0.0
1	C	5.7	0.0	-5.3	0.0	-4.9	0.0	0.1	0.0	-0.1	0.0	4.8	0.0	3.8	0.0
1	B	4.8	0.0	-4.8	0.0	-8.9	0.0	0.0	0.0	0.0	0.0	5.7	0.0	0.8	0.0
1	A	2.2	0.0	-1.9	0.0	-3.1	0.0	0.0	0.0	0.0	0.0	2.1	0.0	0.5	0.0

Frm Line	Col Line	E1UNB_SL_R-- Horz	E1UNB_SL_R-- Vert	E1PAT_LL_1-- Horz	E1PAT_LL_1-- Vert	E1PAT_LL_2-- Horz	E1PAT_LL_2-- Vert	E1PAT_LL_3-- Horz	E1PAT_LL_3-- Vert	E1PAT_LL_4-- Horz	E1PAT_LL_4-- Vert	E1PAT_LL_5-- Horz	E1PAT_LL_5-- Vert
1	E	0.0	0.5	0.0	1.9	0.0	-0.2	0.0	0.0	2.2	0.0	-0.3	0.0
1	D	0.0	0.9	0.0	5.6	0.0	2.2	0.0	-0.2	0.0	2.6	0.0	2.7
1	C	0.0	3.8	0.0	2.3	0.0	5.5	0.0	2.3	0.0	2.3	0.0	2.3
1	B	0.0	4.8	0.0	-0.3	0.0	2.2	0.0	5.6	0.0	2.7	0.0	2.6
1	A	0.0	1.3	0.0	0.0	0.0	-0.2	0.0	1.9	0.0	-0.3	0.0	2.2

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert
6	A	0.4	0.3	2.0	2.1	0.0	-2.9	0.0	-2.3	0.0	-1.7	0.0	-1.1
6	B	1.0	0.9	5.4	5.7	0.0	-8.8	0.0	-5.1	0.0	-6.2	0.0	-2.5
6	C	1.0	0.7	4.5	4.8	0.0	-5.5	0.0	-4.7	0.0	-3.9	0.0	-3.1
6	D	1.0	0.9	5.4	5.7	2.5	-7.7	0.0	-6.7	2.5	-5.1	0.0	-4.0
6	E	0.4	0.3	2.0	2.1	0.0	0.7	2.5	-5.5	0.0	1.8	2.5	-4.3

Frm Line	Col Line	Wind_Press Horz	Wind_Press Vert	Wind_Suct Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis_Long Horz	Seis_Long Vert
6	A	-10.0	-5.3	2.2	5.3	0.0	-3.1	0.0	-1.9	0.0	0.0	0.0	0.0	-2.7	-1.8
6	B	-4.3	0.0	4.8	0.0	0.0	-8.9	0.0	-4.8	0.0	0.0	0.0	0.0	0.0	0.0
6	C	-5.1	0.0	5.7	0.0	0.0	-4.9	0.0	-5.3	0.0	-0.1	0.0	0.1	0.0	0.0
6	D	-4.3	0.0	4.8	0.0	0.0	-3.9	1.1	-10.0	0.6	-0.7	0.0	0.6	0.0	0.0
6	E	-10.0	-5.3	2.2	5.3	1.1	-3.0	0.0	-1.8	0.0	0.8	0.6	-0.6	-2.7	-1.8

Frm Line	Col Line	-MIN_SNOW-- Horz	-MIN_SNOW-- Vert	E2UNB_SL_L-- Horz	E2UNB_SL_L-- Vert	E2UNB_SL_R-- Horz	E2UNB_SL_R-- Vert	E2PAT_LL_1-- Horz	E2PAT_LL_1-- Vert	E2PAT_LL_2-- Horz	E2PAT_LL_2-- Vert	E2PAT_LL_3-- Horz	E2PAT_LL_3-- Vert	E2PAT_LL_4-- Horz	E2PAT_LL_4-- Vert
6	A	0.0	2.1	0.0	1.3	0.0	0.5	0.0	1.9	0.0	-0.2	0.0	0.0	0.0	2.2
6	B	0.0	5.7	0.0	4.8	0.0	0.8	0.0	5.6	0.0	2.2	0.0	-0.3	0.0	2.6
6	C	0.0	4.8	0.0	3.8	0.0	3.8	0.0	2.3	0.0	5.5	0.0	2.3	0.0	2.3
6	D	0.0	5.7	0.0	0.9	0.0	4.8	0.0	-0.2	0.0	2.2	0.0	5.6	0.0	2.7
6	E	0.0	2.1	0.0	0.5	0.0	1.3	0.0	0.0	0.0	-0.2	0.0	1.9	0.0	-0.3

Frm Line	Col Line	E2PAT_LL_5-- Horz	E2PAT_LL_5-- Vert
6	A	0.0	-0.3
6	B	0.0	2.7
6	C	0.0	2.3
6	D	0.0	2.6
6	E	0.0	2.2

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	E	7	1.3	-3.1	8	-1.1	-1.6	4	0.625	6.000	6.000	0.250	0.0
		9	1.0	3.1	7	1.3	-3.1						
1	D	10	2.9	-5.4	11	-2.6	-5.4	4	0.625	6.000	6.000	0.250	0.0
		6	0.0	7.5	10	2.9	-5.4						
1	C	12	3.4	-2.7	11	-3.1	-2.6	4	0.625	6.000	6.000	0.250	0.0
		13	0.0	7.2	12	3.4	-2.7						
1	B	14	2.9	-4.7	8	-2.6	-4.7	4	0.625	6.000	6.000	0.250	0.0
		6	0.0	7.5	14	2.9	-4.7						
1	A	14	1.3	-1.7	8	-1.1	-1.7	4	0.625	6.000	6.000	0.250	0.0
		15	0.0	2.9	14	1.3	-1.7						
6	A	10	1.3	1.5	11	-6.0	-4.8	4	0.625	6.000	6.000	0.250	0.0
		9	1.0	4.1									
6	B	10	2.9	-4.7	11	-2.6	-4.7	4	0.625	6.000	6.000	0.250	0.0
		6	0.0	7.5	10	2.9	-4.7						
6	C	7	3.4	-2.7	8	-3.1	-2.6	4	0.625	6.000	6.000	0.250	0.0
		16	0.0	7.2	7	3.4	-2.7						
6	D	14	2.9	-5.4	8	-2.6	-5.4	4	0.625	6.000	6.000	0.250	0.0
		6	0.0	7.5	14	2.9	-5.4						
6	E	12	1.3	0.1	11	-6.0	-4.7	4	0.625	6.000	6.000	0.250	0.0
		17	1.0	5.4									

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.
 - Width (ft) : 80
 - Length (ft) : 125
 - Eave Height (ft) : 18 / 18
 - Roof Slope (rise/12) : 2.0:12 / 2.0:12
 - Design Code : IBC 15
 - Enclosure : Closed
 - Dead Load (psf) : 2.000
 - Collateral Load (psf) : 3
 - Ultimate Design Wind Speed (mph) : Vult (3 sec. gust) = 115.00 mph
 - Nominal Design Wind Speed (mph) : Vasd (3 sec. gust) = 89.08 mph
 - Wind Importance Factor : 1.00
 - Wind Exposure : C
 - Live Load (psf) : 20.00
 - Frame Live Load (psf) : 12
 - Ground Snow Load (psf) : 20.000
 - Roof Snow Load (psf) : 20.000
 - Snow Exposure : 1.000
 - Snow Importance Factor : 1.000
 - Thermal Factor : 1.000
 - Seismic Importance Factor : 1.00
 - Spectral Response Accel. : Ss=0.194 : S1=0.104
 - Spectral Response Coeff. : Sds=0.207 : Sd1=0.165
 - Seismic Coeff. (Fa*Ss) : 0.310 : Fa=1.600
 - Seismic Design Category : C
- Loading conditions are:
 - Dead+Collateral+Snow+Slide_Snow
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Right1
 - 0.6Dead+0.6Wind_Long1L
 - 0.6Dead+0.6Wind_Long2L
 - Dead+Collateral+MIN_SNOW
 - 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - Dead+Collateral+0.45Wind_Right2+0.45Wind_Suction+0.75MIN_SNOW
 - 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
 - 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 - Dead+Collateral+E1PAT_LL_2
 - 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
 - Dead+Collateral+E1PAT_LL_5
 - Dead+Collateral+E2PAT_LL_2
 - Dead+Collateral+0.45Wind_Left2+0.45Wind_Suction+0.75MIN_SNOW

BUILDING BRACING REACTIONS

Wall Loc	Col Line	Panel Shear (lb/ft)	± Reactions(k)			
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert
L_EW	1	E,D	2.5	2.6	0.6	0.7
F_SW	A	5,6	8.0	5.3	2.7	1.8
R_EW	6	D,E	2.5	2.6	0.6	0.7
B_SW	E	6,5	8.0	5.3	2.7	1.8

FOR CONSTRUCTION

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	ANCHOR BOLT REACTIONS
0	PERMIT/CONSTRUCTION	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstown
						END USER	City of Bardstown
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstown, KY 40004
						SCALE	70112 152536 N.T.S. F004



GENERAL NOTES

1.1 Fabrication shall be in accordance with D.B.S.B. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3". D.B.S.B. manufacturing procedures are certified by:

Reference	Certification numbers
Houston	D.B.S.B. #456

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, S, C & L)	A572/A529	Fy = 50 KSI
Hot Rolled Steel Shapes (W)	A992	Fy = 50 KSI
Round Structural Tubing (HSS)	A500	Fy = 42 KSI
Square/Rect. Structural Tubing (HSS)	A500	Fy = 46 KSI
Structural Steel Web Plate	A572/A1011	Fy = 55 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475	Extra High Strength
Rod Brace	A36	Fy = 36 KSI

		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1" and less)	A325-TYPE 1	Fu = 120 KSI
High Strength Bolts (>1" to 1 1/2")	A325-TYPE 1	Fu = 105 KSI
Anchor Bolts (if supplied)	A36/A307/F1554 Gr.36	Fu = 58-80 KSI

1.3 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is D.B.S.B. Red Oxide color. This paint is not intended for long term exposure to the elements. D.B.S.B. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. D.B.S.B. shall not be responsible for any field applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 14th Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 GALVANIZED OR SPECIAL COATINGS:

See Contract Documents

1.5 ALL BOLTS ARE 1/2"Ø x 0'-1" A307 (snug-tightened) EXCEPT:

- a) Eave strut connection - 1/2"Ø x 0'-1 1/4" A307 without washer (unless noted otherwise)
- b) Endwall rafter splice - 5/8"Ø x 0'-1 3/4" A325-N with washer
- c) Endwall column to rafter connection - 1/2"Ø x 0'-1 1/4" A325-N without washer
- d) Main frame moment splice connections - A325-N with washer, SEE CROSS SECTION for dimensions.

NOTE: One (01) washer is supplied on main frame moment splice and to A325 bolts unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325-N unless specifically noted otherwise. Structural bolts shall be tightened by the turn-of-the-nut or calibrated wrench methods in accordance with the 14th Edition AISC/RCS "Specification For Structural Joints using ASTM A325 or A490 Bolts". Washers are supplied separately from High Strength Bolts, however, assembly with washers are required before erection. Installation inspection is recommended and be based on Section 9.1 and 9.2 of AISC/RCS.

Snug-tight is permitted EXCEPT for the following conditions:

- a) Building located in high seismic areas; Seismic Design Categories D, E, F
- b) Building supporting cranes
- c) Building supporting machinery that creates vibration, impact or stress reversal
- d) Connections using ASTM A490
- e) Connections using slip-critical condition
- f) or as prohibited in the contracts/specifications

1.7 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION:

INSIDE - Under roof panels at eave
OUTSIDE - Between endwall panels and rake trim
- Under continuous ridge vent skirts

1.8 ERECTION NOTE:

All bracing, strapping, & bridging shown and provided by D.B.S.B. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 ERECTION AND UNLOADING NOT BY D.B.S.B.

1.10 SHORTAGES

Any claims or shortages by buyer must be made to D.B.S.B. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)

Claims for correction of alleged misfits will be disallowed unless D.B.S.B. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of D.B.S.B.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release D.B.S.B. to fabricate upon receiving such.

2.2 Duro Beam Steel Buildings (hereafter referred to as D.B.S.B.) standard specifications apply unless stipulated otherwise in the Contract Documents. D.B.S.B. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between D.B.S.B. structural steel plans and plans for other trades, D.B.S.B. plans shall govern. (Section. 3 AISC Code of Standard Practices, 14th Edition)

2.4 Approval of D.B.S.B. drawings and calculations indicates that D.B.S.B. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the D.B.S.B. design concepts, assumptions, and loading. (Section 4 AISC Code 14th Edition and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed D.B.S.B. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.



DRAWING PACKAGE

SALES NO.	70112	JOB NO.	152536	BUILDING	A
CUSTOMER	City of Bardstown				
END USER	City of Bardstown				
END USE	Cable Building				
STREET	999 Kelly Drive				
CITY ST ZIP	Bardstown, KY 40004				
COUNTY	Nelson				

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code	: IBC 15
Dead Load (psf)	: Metal building structure only by RGB
Collateral Load (psf)	: 3
Wind Load	
Ultimate Design Wind Speed	: Vult (3 sec. gust) = 115 mph
Nominal Design Wind Speed	: Vasd (3 sec. gust) = 89.080
Risk Category	: II - Normal
Wind Exposure	: C
Enclosure	: Closed
Internal Pressure Coefficient, GCPi	: 0.180 / -0.180
Design Wind Pressure For Wall	: Based on Nominal Design Wind Speed
Components Wind Pressure (psf) asd	: 13.98
Components Wind Suction (psf) asd	: -15.36
Claddings Wind Pressure (psf) asd	: 16.44
Claddings Wind Suction (psf) asd	: -17.76
Live Load	
Primary Framing (psf)	: 20.00
Trib. Area Reduction	: Yes
Secondary Framing (psf)	: 20.00
Snow Load	
Ground Snow Load, Pg (psf)	: 20.000
Roof Snow Load, Pf (psf)	: 20.000
Sloped Roof Snow Load, Ps (psf)	: 20.000
Snow Exposure Factor, Ce	: 1.000
Snow Importance Factor, Is	: 1.000
Thermal Factor, Ct	: 1.000
Sloped Factor, Cs	: 1.000
Seismic Load	
Seismic Importance Factor, Ie	: 1.00
Seismic Occupancy Category	: II - Normal
Site Class	: D
Mapped Spectral Response Acceleration	: Ss = 0.194 :S1 = 0.104
Spectral Response Coefficients	: Sds = 0.207 :Sd1 = 0.165
Seismic Design Category	: C
Basic Force Resisting Systems Used	: Steel Systems Not Specifically Detailed For Seismic Resistance
	: Rigid Frames
	: Braced Frames
Total Design Base Shear, V (kips)	: Longitudinal= 5.38 Transverse= 5.36
Response Modification Factors, R	: Rigid Frames = 3.00
	: SW X-Bracing = 3.00
	: EW X-Bracing = 3.00
Seismic Response Coefficient, Cs	: Rigid Frames = 0.069
	: SW X-Bracing = 0.069
	: EW X-Bracing = 0.069
Analysis Procedure Used	: Equivalent Lateral Force Procedure
Rainfall Intensity (in/hr)	: 5.800
Other Loads/Requirements	: None

BUILDING DESCRIPTION:

Width (ft)	: 80
Length (ft)	: 125
Eave Ht. at BSW (ft)	: 18
Eave Ht. at FSW (ft)	: 18
Roof Slope at BSW	: 2.0:12
Roof Slope at FSW	: 2.0:12
Bay Spacing (ft)	: 5 at 25

COVERING AND TRIMS:

Roof Panels & Trims	
Panel Type	: 26 Ga. PBR
Panel Color	: Gvm.Plus
Trim Colors	
Eave Trim	: Koko Brown
Eave Gutter	: NONE
Gable Trim	: Koko Brown
Wall Panel & Trims	
Panel Type	: 26 Ga. PBR
Panel Color	: Lt.Stone
Trim Colors	
Corner Trims	: Koko Brown
Opening Trims	: Koko Brown
Downspouts	: NONE
Base Trim	: ---
Mas. Flash	: ---
Special Requirements	: NONE

Before erecting your building, please see the Rigid Erection & Safety Manual at rigidbuilding.com/document-library

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2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by D.B.S.B. and D.B.S.B. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or D.B.S.B. assumptions will govern (Section 4 and Commentary, AISC Code of Standard Practice, 14th Edition)

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to ensure that D.B.S.B. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that D.B.S.B. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by D.B.S.B.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with D.B.S.B. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practice, 14th Edition.)

2.9 Duro Beam Steel Buildings is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete, anchor bolt embedment or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, D.B.S.B. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage/anchor bolt embedment and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Chapter IV Section 3.2.2 Metal Building Systems Manual 2012 Edition)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to D.B.S.B. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (Section 7 AISC Code of Standard Practice, 14th Edition)

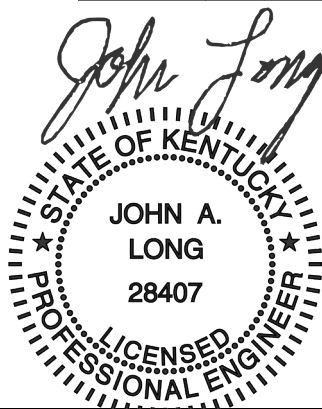
2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 14th Edition)

2.12 WARNING: In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.

2.13 SAFETY COMMITMENT: Duro Beam Steel Buildings has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of D.B.S.B. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
30	17.90	8.95
40	19.20	9.60
50	20.50	10.25
60	21.80	10.90
70	23.10	11.55
80	24.40	12.20



4/13/2021



NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2012 Edition, Section A9.4, Page A-59

City Of Bardstown					
SALES NO.:	70112	JOB NO.:	152536	PACKAGE:	A
				DWG. NO.:	C001
				SCALE:	A

UNLOADING, HANDLING AND STORING OF MATERIALS

STRUCTURAL

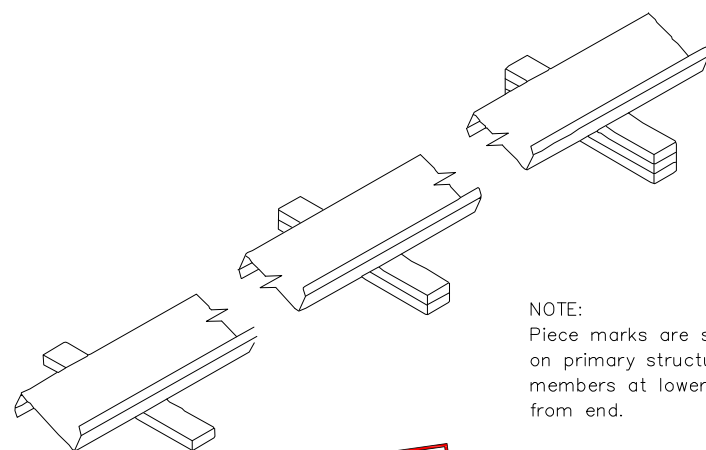
A great amount of time and trouble can be saved if the building site is according to a pre-arranged plan. Proper location and handling of components will eliminate unnecessary handling.

Inspect all shipments prior to releasing the tie-downs for loads that may have shifted during transit, REMEMBER, SAFETY FIRST!

Blocking under the columns and rafters protects the splice plates and the slab from damage during the unloading process. It also facilitates the placing of slings or cables around the members for later lifting and allows members to be bolted together into sub-assemblies while on the ground. Extra care should always be exercised in the unloading operations to prevent injuries from handling the steel and to prevent damage to materials and the concrete slabs.

If water is allowed to remain for extended periods in bundles of primed parts such as girts, purlins etc., the pigment will fade and the paint will gradually soften, reducing the bond to the steel. Therefore, upon receipt of a job, all bundles of primed parts should be stored at an angle to allow any trapped water to drain away and permit air circulation for drying. Puddles of water should not be allowed to collect and remain on columns, rafters or beams for the same reason.

All Primer should be touched up as required before erection!



NOTE:
Piece marks are stenciled on primary structural members at lower end, 1'-0" from end.

FOR PERMIT

WALLS AND ROOF PANELS

RGB's wall and roof panels including color coated, galvalume and galvanized, provide excellent service under widely varied conditions. All unloading and erection personnel should fully understand that these panels are quality merchandise which merit cautious care in handling.

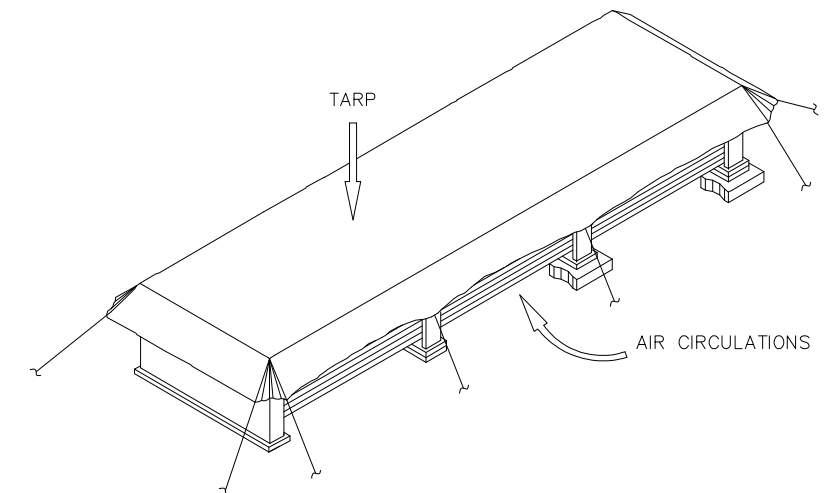
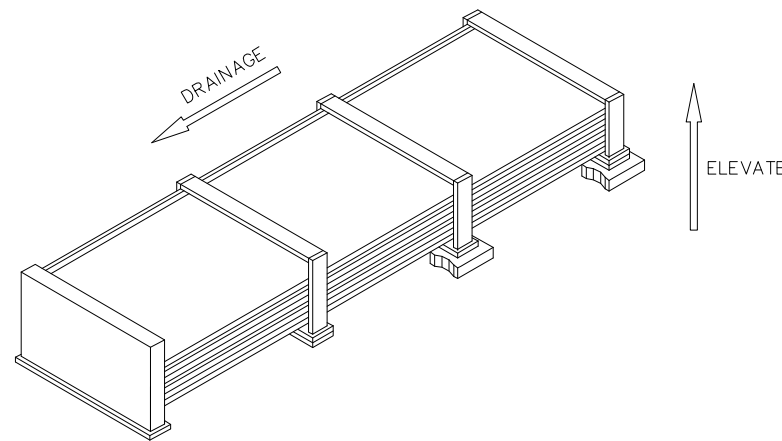
Under no circumstances should panels be handled roughly. Packages of sheets should be lifted off the truck with extreme care taken to insure that no damage occurs to ends of the sheets or to side ribs. The packages should be stored off the ground sufficiently high to allow air circulation underneath the packages. This avoids ground moisture and deters people from walking on the packages. One end of the package should always be elevated to encourage drainage in case of rain.

All stacked metal panels are subject, to some degree, to localized discoloration or stain when water is trapped between their closely nested surfaces. RGB. exercises extreme caution during fabricating and shipping operations to insure that all panel stock is kept dry. However, due to climatic conditions, water formed by condensation of humid air can be trapped between stacked sheets. Water can also be trapped between stacked sheets when exposed to rain. This discoloration caused by trapped moisture is often called wet storage stain.

The stain is usually superficial and has little effect on the appearance or service life of the panels as long as it is not permitted to remain on the panels. However, moisture in contact with the surface of the panels over an extended period can severely attack the finish and reduce the effective service life. Therefore, it is imperative that all panels be inspected for moisture upon receipt of the order. If moisture is present, dry the panels at once and store in a dry, warm place.

CAUTION: Care should always be taken when walking on panels. Use safety lines and nets when necessary! Panels are slippery. Oil or wax applied to the roof and wall panels for protection against weather damage will make them a very slippery surface. Wipe dry any oil that has puddled from bundles stored on a slope. Dew, frost, or other forms of moisture greatly increase the slipperiness of the panels. Always assume panel surface is slippery and act accordingly. Think safety!!

Use wood blocking to elevate and slope the panels in a manner that will allow moisture to drain. Wood blocking placed between bundles will provide additional air circulation. Cover the stacked bundles with a tarp or plastic cover leaving enough opening at the bottom for air to circulate.

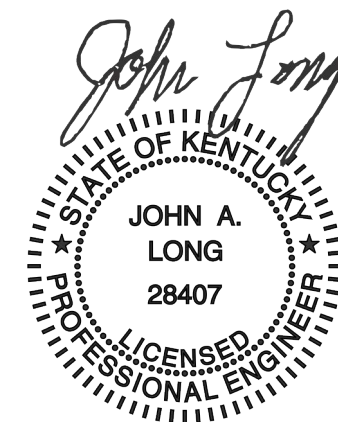


When handling or uncrating the panels, lift, rather than slide, them apart. Burred edges may scratch the coated surfaces when sheets are slid over one another. Never allow panels to be walked on while on the ground.

Rough and improper handling of a panel is inexcusable and a prime example of poor job supervision.

NOTE:
Use gloves when handling metal panels to prevent hand injuries. Be aware, of the dangers of handling panels on a windy day. A large panel can catch enough wind to knock a worker off his feet, even at ground level!! Safety first!

GENERAL NOTE:
1. OIL CANNING OF PANELS IS NOT A CAUSE OF REJECTION.
2. EXTREME CARE MUST BE EXERCISED DURING THE ERECTION OF ROOF PANELS AND TRIMS. FOOT TRAFFIC MAY RESULT IN PERMANENT PANEL DISTORTION AND FINISH ABRASION.



4/13/2021

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CITY OF BARDSTOWN			
SALES NO. 70112	JOB NO. 152536	BUILDING A	DATE: C002 PAGE: A

LEGENDS & ABBREVIATIONS

DESIGN:

Accel.	Acceleration
Coeff.	Coefficient
CL, Collat	Collateral Load
DL, Dead	Dead Load
H, Horz, Horiz	Horizontal
L	Left
LL, Live	Live Load
LnWind, LnWind, LWIND	Longitudinal Wind Load
Min, min	Minimum
Max, max	Maximum
R	Right
SL	Snow Load
Slide	Sliding Snow Load
SEIS, Seis	Seismic Load
U_Snow	Unbalance Snow Load
V, Vert	Vertical
WL, Wind_L	Wind Load Left
WR, Wind_R	Wind Load Right
WP, Wind_P	Wind Pressure
WS, Wind_S	Wind Suction

ENGLISH UNITS

Acre	Acres
FT, ft	Feet
GA, Ga, ga	Gage
Gal	Gallons
IN, in	Inches
K, k	Kips
KSI, ksi	Kips Per Square-Inches
lb, #	Pounds
MPH, mph	Miles Per Hour
PLF, plf, lb/ft	Pounds Per Linear-Foot
PSF, psf, lb/ft	Pounds Per Square-Foot
TON, ton	Tons
Yd	Yard

METRIC UNITS

cm	Centimeters
Hec	Hectares
liter	Liters
m	Meters
mm	Millimeters
N	Newtons
km	Kilometers
kN	Kilonewtons
kN/m ²	Kilonewtons Per Square-meter
kPa	Kilopascals
kph	Kilometers Per Hour
Pa	Pascals

USEFUL CONVERSION

English	To	English	Metric	To	Metric
1 mile	1760 Yd	1 km	1000 m		
1 Yd	3 Ft	1 m	100 cm		
1 Ft	12 in	1 cm	10 mm		
1 in	16/16 in	1 kN	1000 N		
1 Ton (English)	2 Kips	1 kg	9.8066 N		
1 Kip	1000 lb	1 Ton (Metric)	1000 kg		
1 lb	16 ounces	1 Hec	10,000 m		
1 Acre	43560 ft ²	1 m ³	1000 liter		
1 Ft ²	7.4805 Gal	1 kPa	1 kN/m ²		

English	To	Metric	Metric	To	English
1 in	2.54 cm	1 cm	0.3937 in		
1 ft	0.3048 m	1 m	3.2808 ft		
1 lb	0.4536 kg	1 kg	2.2046 lb		
1 Ton (English)	907.18 kg	1 Ton (Metric)	2204.6 lb		
1 Kip	4.4482 kN	1 kN	0.2248 kip		
1 mile	1.6093 km	1 km	0.6213 mile		
1 Acre	0.4046 Hec	1 Hec	2.4715 Acres		
1 lb/ft ²	0.0478 kPa	1 kPa	20.8854 lb/ft ²		

Fraction	To	Decimal	Fraction	To	Decimal
1/16	0.0625	9/16	0.5625		
1/8	0.1250	5/8	0.6250		
3/16	0.1875	11/16	0.6875		
1/4	0.2500	3/4	0.7500		
5/16	0.3125	13/16	0.8125		
3/8	0.3750	7/8	0.8750		
7/16	0.4375	15/16	0.9375		
1/2	0.5000	16/16	1.0000		

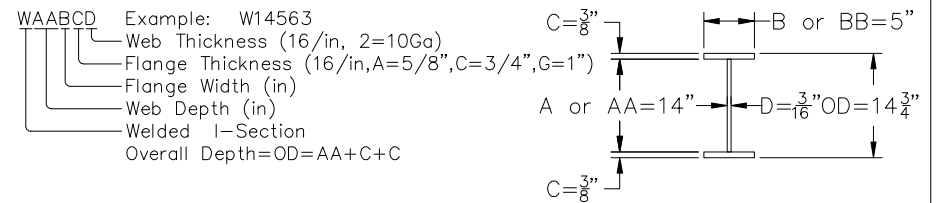
DRAWINGS:

AB, A.B.	Anchor Bolt
AS, As Shown	As Shown
Aux.	Auxiliary
BLDG., Bldg.	Building
B.P., Base PL	Base Plate
BOTT., Bott.	Bottom
Bott. Base PL, B.O.B.P	Bottom Of Base Plate
B.O.S.	Bottom Of Steel
BSW	Back Sidewall
BY OTHERS	By Other Supplier or Not By Rigid
C/C	Center to Center
C.I.P.	Cast-In-Place
CL, C	Center Line
CLR.	Clear, Clearance
CMU	Concrete Masonry Unit
COL., Col.	Column
CONC., Conc.	Concrete
CONT.	Continuous, Continuation
DET.	Detail
DIA., Dia./O	Diameter
DIM., Dim.	Dimension
DWG., Dwg.	Drawing
EH, E.H.	Eave Height
EJ, Exp. Jt.	Expansion Joint
EL, Elev.	Elevation
EP	End Plate
ES, E.S.	Eave Strut
EW	Endwall
EW COL, EC	Endwall Column
EW RAF	Endwall Rafter
Exp. Bolt	Expansion Bolt
FFL, Fin. Flr.	Finish Floor Line
FLG., FLGE., Flg., Flge.	Flange
FNB, F.N.B.	Fin Neck Bolt
FO, F.O.	Framed Opening
FRM., Frm.	Frame
FSW	Front Sidewall
GA, Ga.	Gage
GALV., Galv.	Galvanized
G.O.L.	Gage of Outstanding Leg
H, Ht.	Height
HED, HEDS	High Eave Double Slope
HES, HESS	High Eave Single Slope
Horz, Horiz	Horizontal
HSB, H.S.B.	High Strength Bolt
HSS	Hollow Structural Section
INT., Int.	Interior, Intermediate
I/S	Inside
LED, LEDS	Low Eave Double Slope
LES, LESS	Low Eave Single Slope
LEW	Left Endwall
LHI	Left Hand In
LHO	Left Hand Out
LL	Long Life
LLH	Long Leg Horizontal
LLV	Long Leg Vertical
LT	Lean-To
LT COL	Lean-To Column
LT RAF	Lean-To Rafter
LG., Lg.	Long
L, Lt.	Length
L x W x H	Length x Width x Height
MAX., max.	Maximum
MIN., min.	Minimum
MKD., MK'D.	Marked
MB, M.B.	Machine Bolt
MEZZ., Mezz.	Mezzanine
N.A., N/A	Not Applicable
NO., No.	Number
NS/FS, NS&FS	Near Side and Far Side
O.C.	On Center
O/S	Outside
OH, Opp Hand	Opposite Hand (Mirror Image)
OHD, O.H.D.	Over-Head Door
O/O	Out to Out
PF COL	Portal Frame Column (Wind Bent Column)
PF RAF	Portal Frame Rafter (Wind Bent Rafter)
PL, p	Plate
QTY., Qty.	Quantity
REF., Ref.	Refer, Reference
REW	Right Endwall

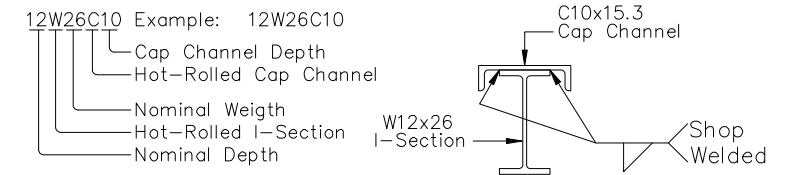
DRAWINGS:

RHB	Round Head Bolt
RHI	Right Hand In
RHO	Right Hand Out
REINF.	Reinforced
REQ'D., REQD.	Required
REV., Rev.	Revised, Revision
RF, R.F.	Rigid Frame
RF COL	Rigid Frame Column
RF RAF	Rigid Frame Rafter
RUD, R.U.D.	Roll-Up Door
SC	Slip Critical
SDS	Self-Drilling Screws
SECT., Sect.	Section
SHTG., Shtg.	Sheeting
Sol Col	Soldier Column
SP	Splice Plate
SSR	Standing Seam Roof
SST	Stainless Steel
ST COL	Straight Column
STIFF.	Stiffener
STD.	Standard
STS	Self-Tapping Screws
SW	Sidewall
SYM., Sym., SYMM., Sym	Symmetry, Symmetrical
TBE	To Be Established
TBD	To Be Determined
TC	Tension Control
THK., Thk.	Thick
TOC, T.O.C.	Top Of Concrete
TOS, T.O.S.	Top Of Steel
T & B, TOP & BOTT	Top and Bottom
TYP., Typ., typ.	Typical
UN, U.N.O.	Unless Noted, Unless Noted Otherwise
Vert.	Vertical
WD	Walk Door
W, Wd.	Width
W.P.	Work Point, Working Point

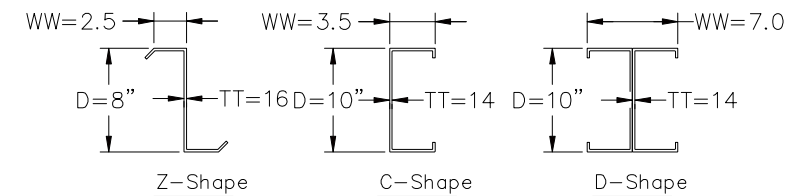
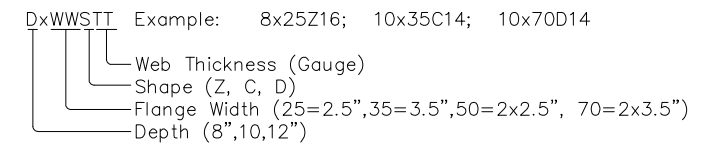
BUILT-UP SECTIONS: USED FOR FRAMES, BEAMS, COLUMNS



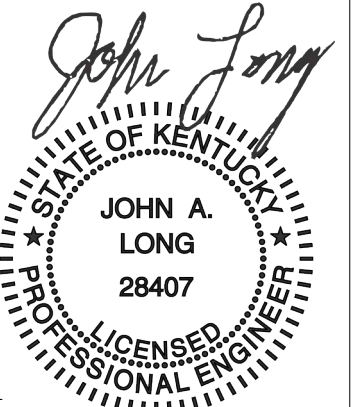
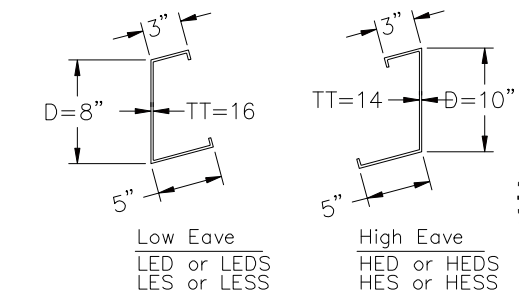
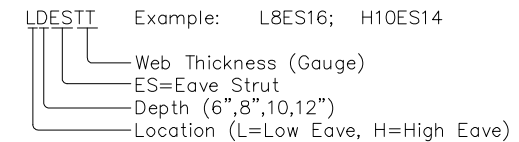
CRANE BEAM SECTIONS: USED FOR RUNWAY BEAMS, MONORAILS



COLD-FORMED SECTIONS Z,C,D: USED FOR PURLINS, GIRTS, JAMBS, JOISTS

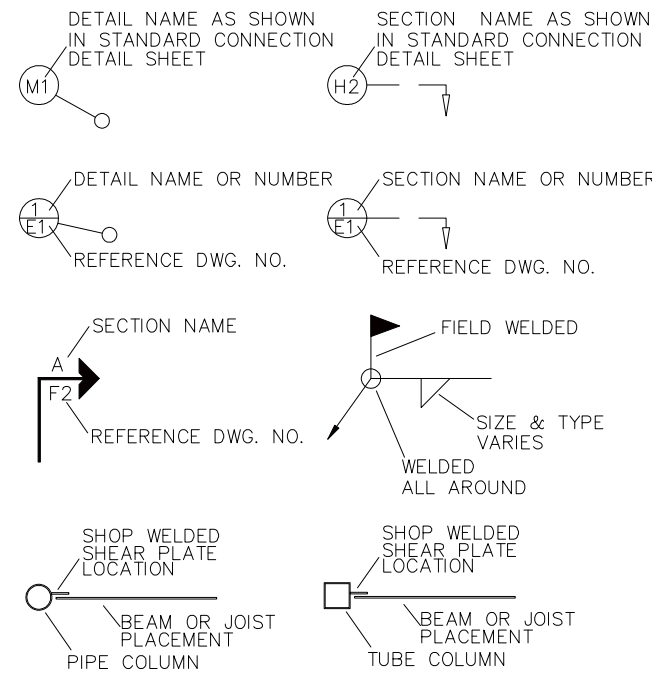


COLD-FORMED SECTION ES: USED FOR EAVE STRUTS



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SECTIONS AND DETAILS:



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB



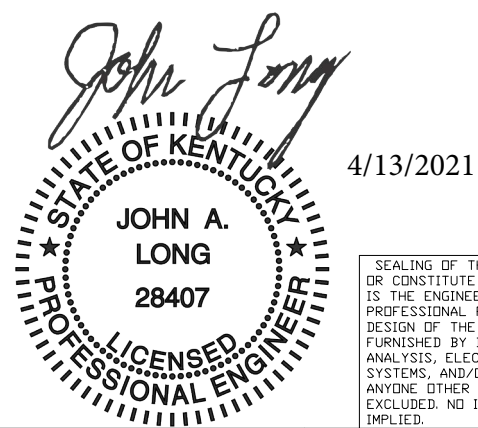
DESCRIPTION	LEGENDS & ABBREVIATIONS
CUSTOMER	City of Bardstovwn
END USER	City of Bardstovwn
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstovwn, KY 40004
SALES NO.	152536
SCALE	N.T.S.
DWG. NO.	C003
ISSUE	A



DRAWING INDEX

DWG.NO.	ISSUE	DRAWING TITLE	DWG.NO.	ISSUE	DRAWING TITLE	DWG.NO.	ISSUE	DRAWING TITLE
C001	A	COVER SHEET						
C002	A	UNLOADING, HANDLING & STORAGE OF MATERIALS						
C003	A	LEGENDS AND ABBREVIATIONS						
C004	A	DRAWING INDEX						
F001	0	COLUMN LAYOUT PLAN						
F002	0	ANCHOR BOLT DETAILS						
F003	0	ANCHOR BOLT REACTIONS & DESIGN CALCULATION WIND						
F004	0	ANCHOR BOLT REACTIONS & DESIGN CALCULATION WIND						
E001	A	ROOF FRAMING PLAN						
E002	A	ROOF SHEETING						
E003	A	RIGID FRAME ELEVATION						
E004	A	RIGID FRAME ELEVATION						
E005	A	ENDWALL, SHEETING & TRIMS						
E006	A	ENDWALL, SHEETING & TRIMS						
E007	A	SIDEWALL, SHEETING & TRIMS						
E008	A	SIDEWALL, SHEETING & TRIMS						
E009	A	DETAIL DRAWINGS						
E010	A	DETAIL DRAWINGS						
E011	A	DETAIL DRAWINGS						
E012	A	DETAIL DRAWINGS						

FOR PERMIT



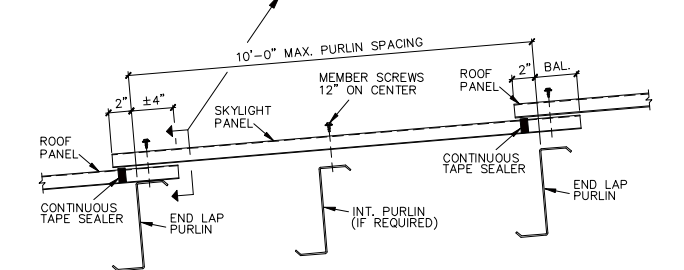
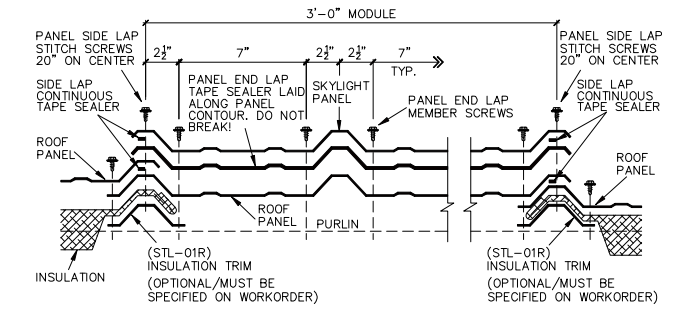
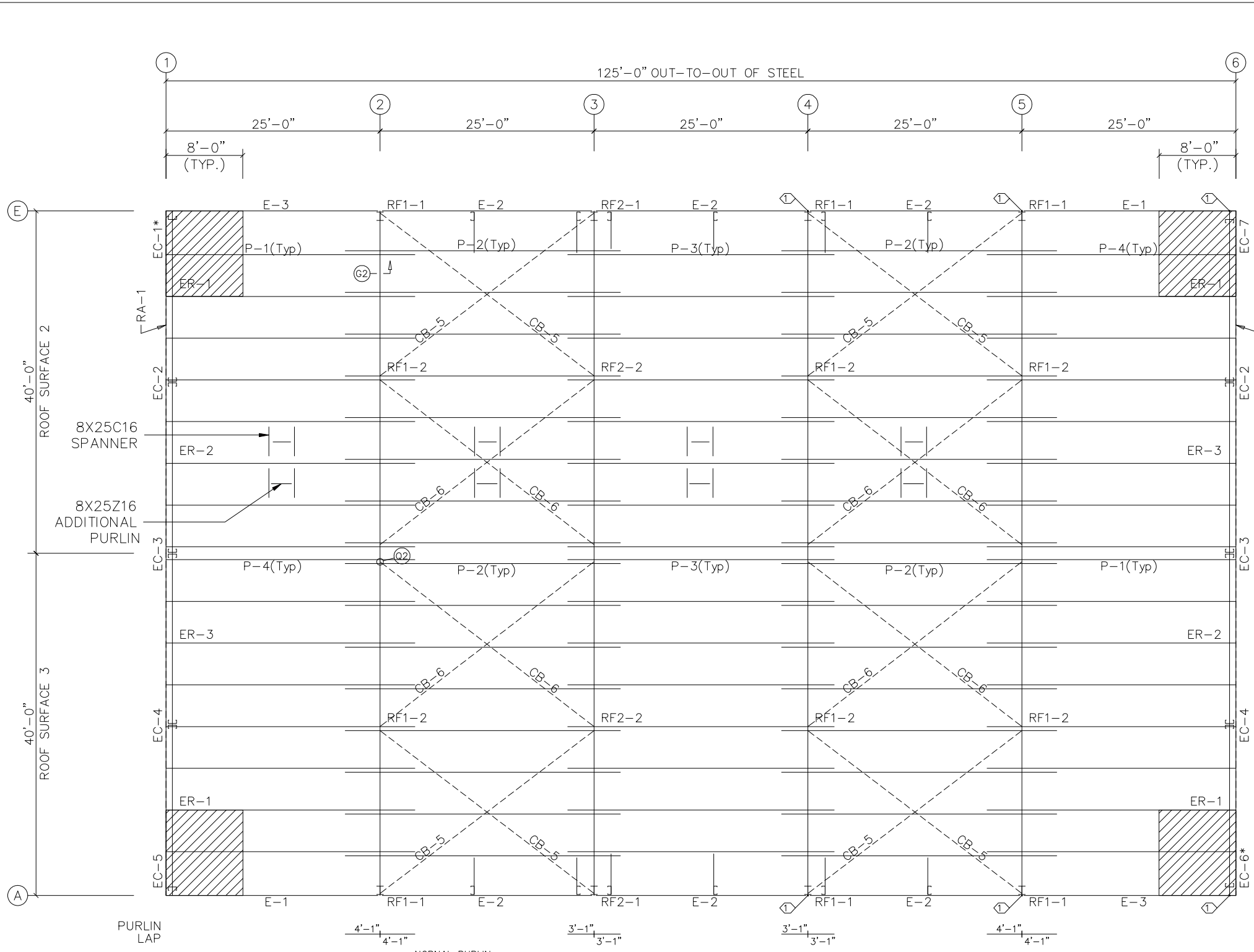
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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	DRAWINGS INDEX
A	PERMIT	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstown
						END USER	City of Bardstown
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstown, KY 40004
						SALES NO.	70112
						JOB NO.	152536
						SCALE	N.T.S.
						DWG. NO.	C004
						ISSUE	A

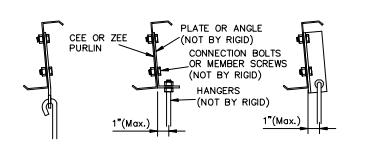


SPECIAL BOLTS					
ROOF PLAN					
Q ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A307	1/2"	1 1/4"	0

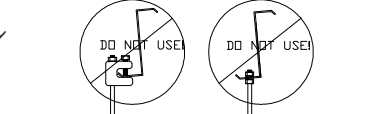
MEMBER TABLE	
ROOF PLAN	
MARK	PART
P-1	8x25Z12
P-2	8x25Z16
P-3	8x25Z14
P-4	8x25Z12
E-1	L8ES16
E-2	L8ES16
E-3	L8ES16
CB-5	CB0250
CB-6	CB0250



STANDARD SKYLIGHT INSTALLATION WITH "R" AND "PBR" ROOF PANEL



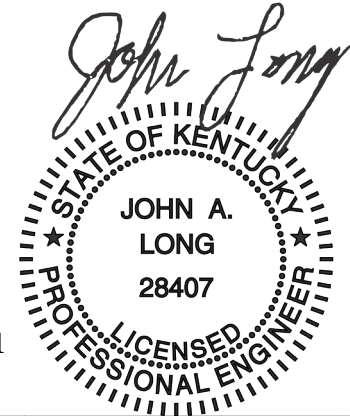
HANGERS ON CEE OR ZEE PURLINS MUST ATTACH TO THE WEB ONLY. HANGERS MUST BE LOCATED SO THAT THE SUPPORTED LOAD DOES NOT EXCEED THAT SPECIFIED IN THE CONTRACT DOCUMENTS. SUGGESTED DETAILS ARE AS SHOWN ABOVE.



DO NOT USE C-CLAMPS OR ANY OTHER HANGER ATTACHED TO THE FLANGE OF THE PURLIN. IT REDUCES THE CAPACITY OF THE PURLIN AND PRODUCES SECONDARY STRESSES.

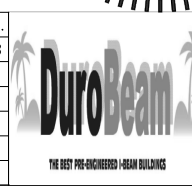
1 TYP. COLLATERAL LOAD CONNECTION DETAIL

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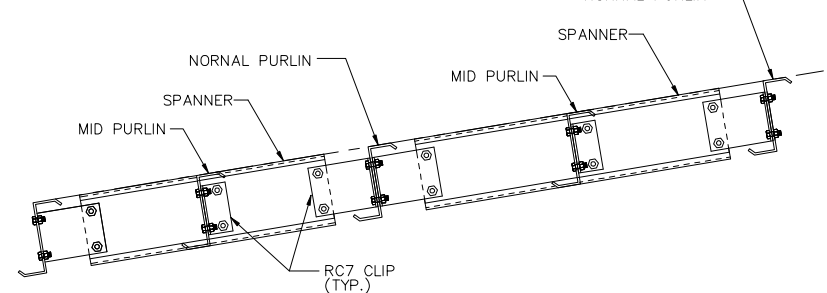
4/13/2021

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
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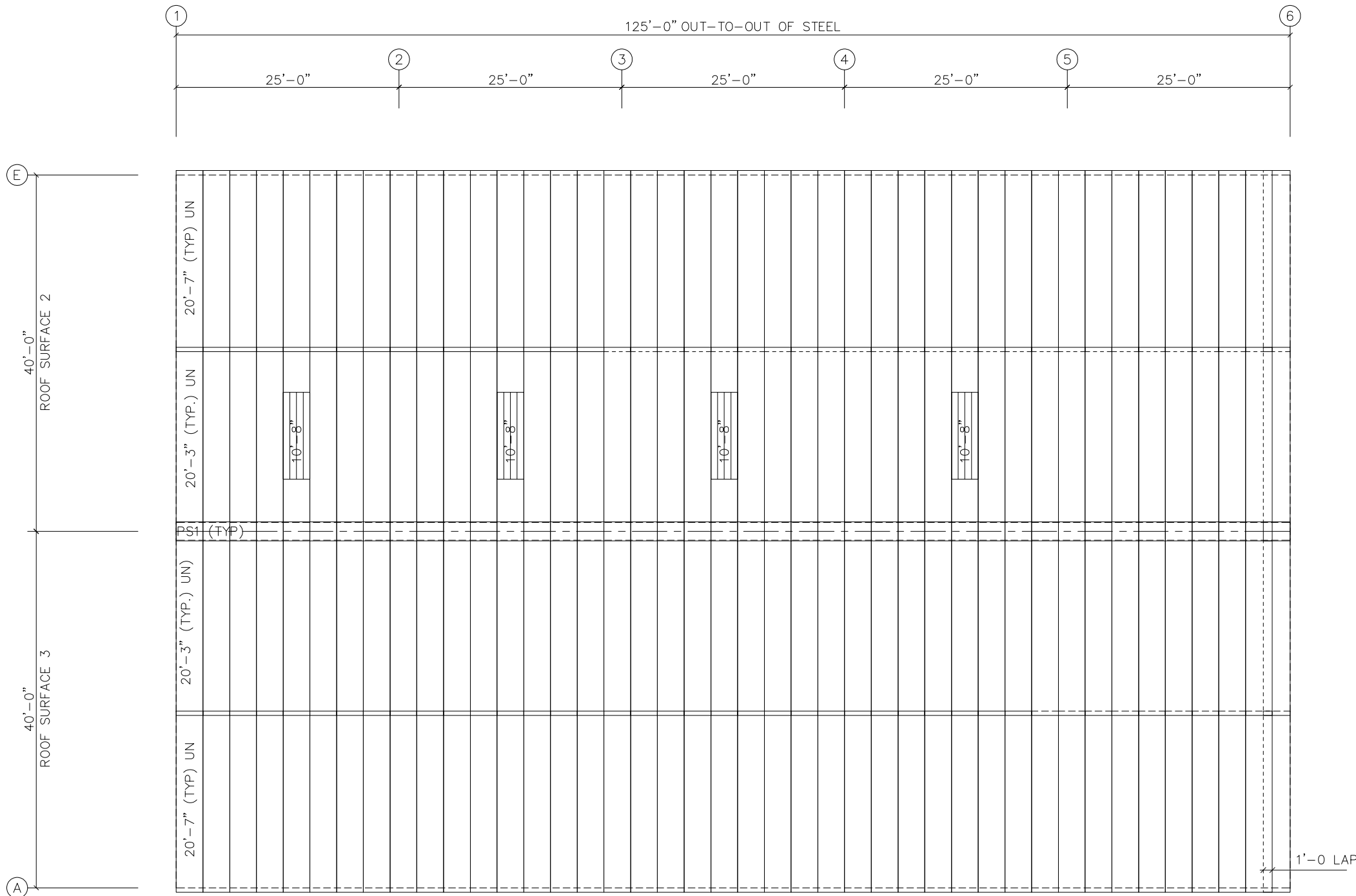


DESCRIPTION	ROOF FRAMING PLAN
CUSTOMER	City of Bardstovwn
END USER	City of Bardstovwn
END USE	Cable Building
STREET	999 Kelly Drive
CITY ST ZIP	Bardstovwn, KY 40004
SCALE NO.	152536
SCALE	N.T.S.
DWG. NO.	E001
ISSUE	A

ROOF FRAMING PLAN
 NOTE: 1. USE 6" MEMBER SCREW SPACING ON SHADED AREA
 2. FIELD LOCATED SPANNERS & ADDITIONAL PURLINS WILL BE LOCATED WHERE SKYLIGHTS WILL BE PLACED.
 3. WITH 6" THK OS INSULATION (BY OTHERS)



SPANNER CONNECTION DETAIL
 ALL BOLTS ARE 1/2" x 1" A307 U.N.



ROOF SHEETING PLAN
 PANELS: 26 Ga. PBR - Glvm.Plus

IMPORTANT NOTES:

- =====
- OIL CANNING OF PANELS IS NOT A CAUSE OF REJECTION.
 - EXTREME CARE MUST BE EXERCISED DURING ERECTION OF ROOF PANELS AND TRIMS. FOOT TRAFFIC MAY RESULT IN PERMANENT PANEL DISTORTION AND FINISH ABRASION.
- =====
- ERECTOR'S NOTE
- =====
- INSTALLER OF STANDING SEAM ROOF PANEL MUST STUDY THE INSTALLATION MANUALS PRIOR TO INSTALLATION. MANUALS ARE PROVIDED WITH THE MATERIALS SHIPMENT BUT CAN BE REQUESTED OR DOWNLOADED FROM THE RIGID GLOBAL BUILDINGS WEBSITE AT www.rigidbuilding.com
 - FAILURE TO INSTALL THE ROOF SHEETS IN ACCORDANCE WITH THE SHEETING DIRECTIONAL ARROWS SHOWN ON THESE PLANS MAY RESULT IN IMPROPER FIT-UP OF THE OUTSIDE CLOSURES (END DAMS) AND POSSIBLY OTHER TRIM COMPONENTS WHICH COULD AFFECT THE OVERALL APPEARANCE AND WEATHER TIGHTNESS OF THE BUILDING. RIGID WILL NOT BE HELD RESPONSIBLE FOR THE CHARGES OR ADDITIONAL FIELD WORK DUE TO NOT FOLLOWING SHEETING DIRECTIONAL ARROWS AND OTHER PROCEDURES OUTLINED IN THE ERECTION MANUAL.
 - IN THE EVENT THAT A DISCREPANCY OR ERROR ARISES WITH MATERIALS SHIPPED FOR THIS PROJECT OR ON THESE ERECTION DRAWINGS, THE ERECTOR/INSTALLER MUST NOTIFY RGB PRIOR TO CORRECTING. IF RGB IS NOT NOTIFIED, RGB WILL NOT HONOR BACKCHARGES BY ANY PARTY INVOLVED.
 - MEMBER SCREW AND STITCH SCREW PATTERNS AND LOCATIONS SHALL BE IN ACCORDANCE WITH ROOF AND WALL DETAILS SHOWN ON DWG.# E011
 - RGB SUPPLIES 5% OVERAGE FOR SCREWS AND ANY CLAIM ON SHORTAGE BECAUSE OF NON-COMPLIANCE WITH THE DRAWINGS SHALL NOT BE RGB'S RESPONSIBILITY.

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4/13/2021

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	ROOF SHEETING PLAN
A	PERMIT	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstown
						END USER	City of Bardstown
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstown, KY 40004
						SCALE NO.	70112 152536 N.T.S. E002



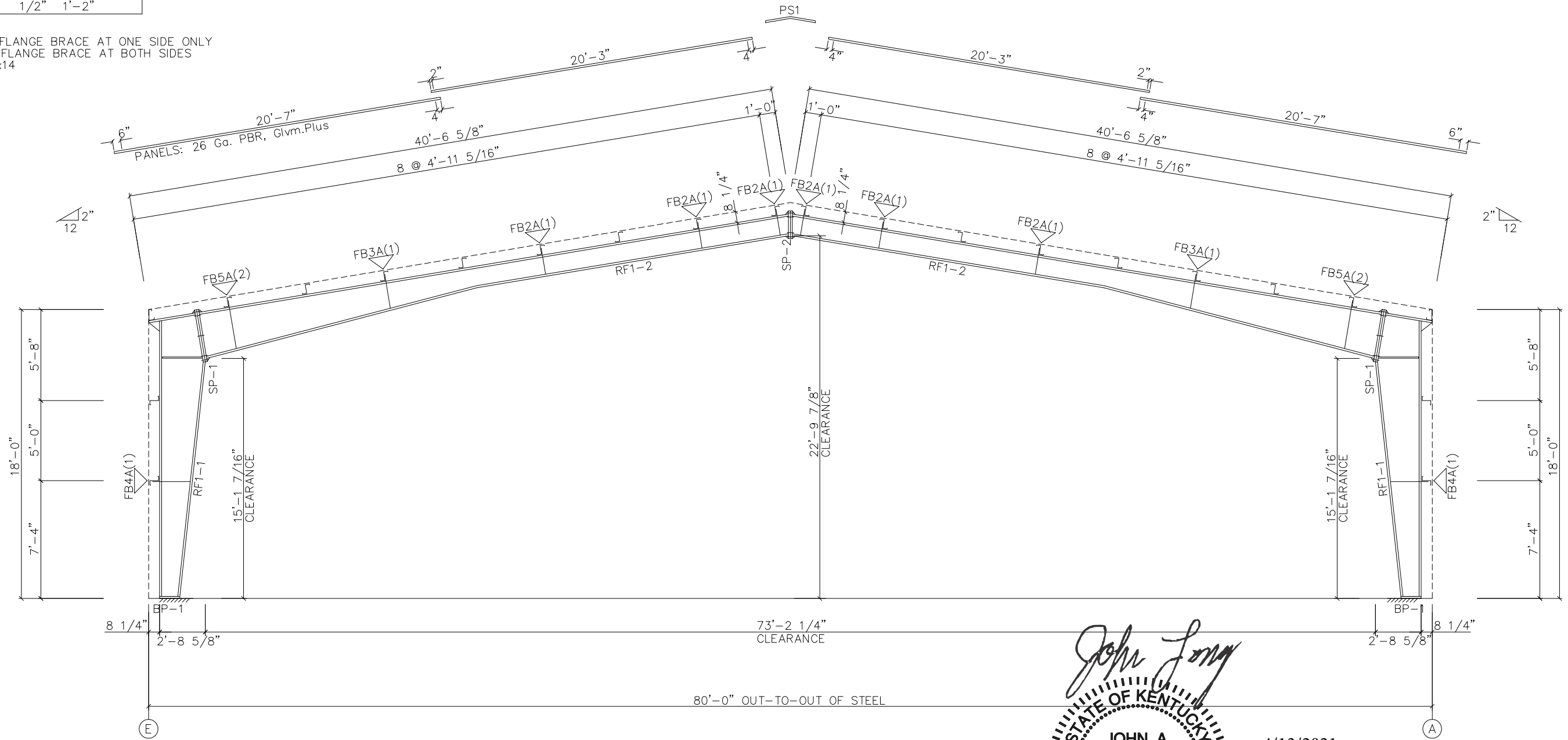
SPLICE PLATE & BOLT TABLE									
Mark	Qty		Type	Dia	Length	Width	Thick	Length	
	Top	Bot						Int	
SP-1	4	4	2	A325	0.875	2.50	6"	5/8"	3'-2 7/8"
SP-2	4	4	0	A325	0.625	2.00	6"	1/2"	1'-7 15/16"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size		
		Width	Thick	Length
RF1-1	St- 1	2.500	0.250	30.97

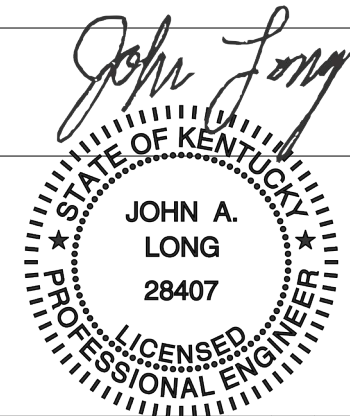
BASE PLATE TABLE			
Col Mark	Plate Size		
	Width	Thick	Length
BP-1	8"	1/2"	1'-2"

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF1-1	12.0	32.0	0.250	177.6	6 x 1/4" x 208.3	6 x 3/8" x 178.7
	32.0	26.2	0.313	35.0	6 x 1/4" x 35.1	
RF1-2	32.0	13.0	0.250	209.7	6 x 1/4" x 209.7	6 x 3/8" x 210.6
	13.0	13.0	0.135	240.0	6 x 5/16" x 240.0	6 x 1/4" x 237.8

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
 FBXXA(2)=FLANGE BRACE AT BOTH SIDES
 A - L2x2x14



RIGID FRAME ELEVATION: FRAME LINE 2 4 5



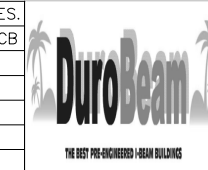
4/13/2021

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NOTE: A325 high strength bolt shall be tightened with one washer. Refer to general notes 1.5 and 1.6 on cover sheet for tightening methods and installation inspections.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	RIGID FRAME ELEVATION
A	PERMIT	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstovnn
						END USER	City of Bardstovnn
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstovnn, KY 40004
						SCALE	1:12
						SCALE	N.T.S.
						SCALE	E003
						SCALE	A



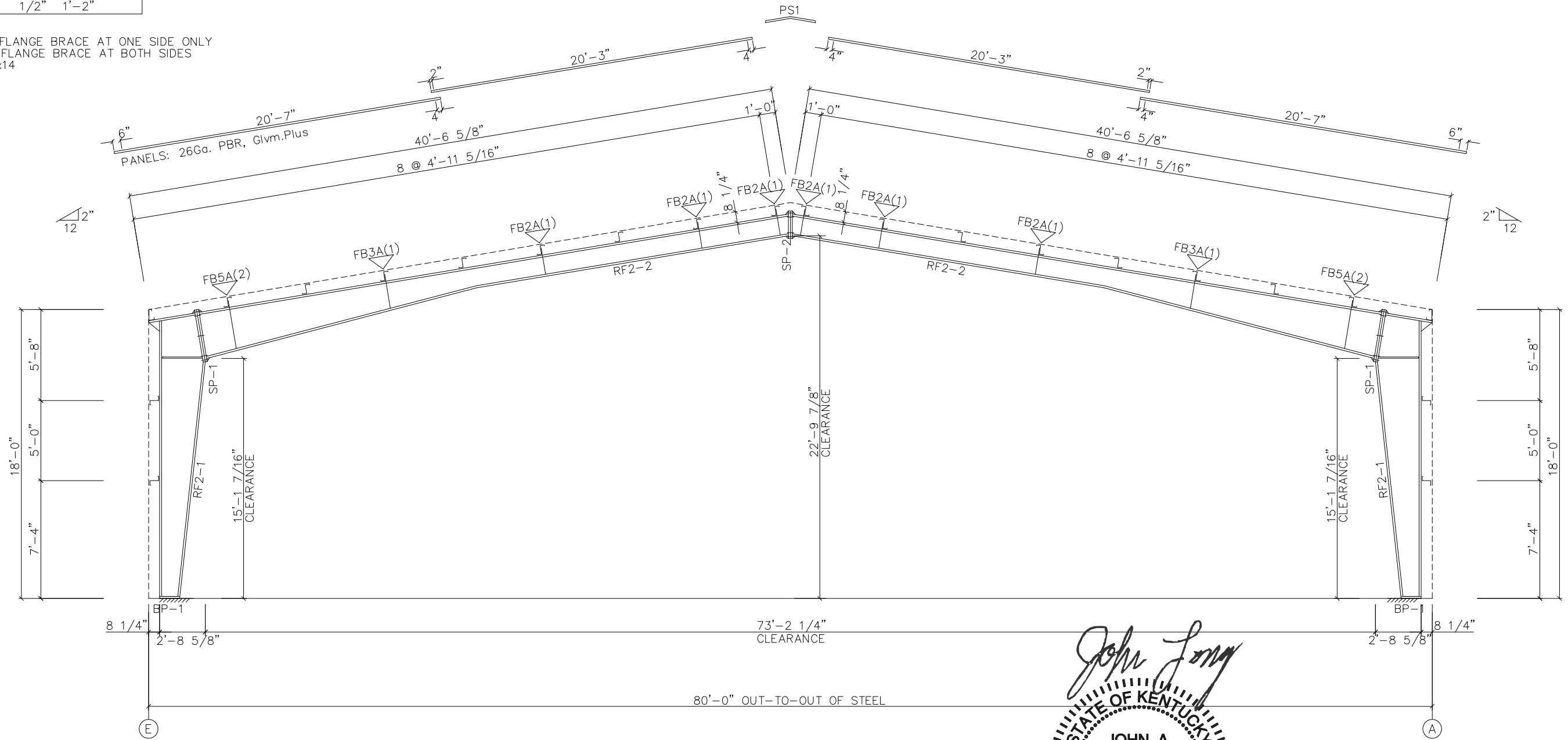
SPLICE PLATE & BOLT TABLE									
Mark	Qty		Type	Dia	Length	Width	Thick	Length	
	Top	Bot						Int	
SP-1	4	4	2	A325	0.875	2.50	8"	5/8"	3'-2 7/8"
SP-2	4	4	0	A325	0.625	2.00	6"	1/2"	1'-7 15/16"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size		
		Width	Thick	Length
RF2-1	St- 1	3.750	0.250	30.97

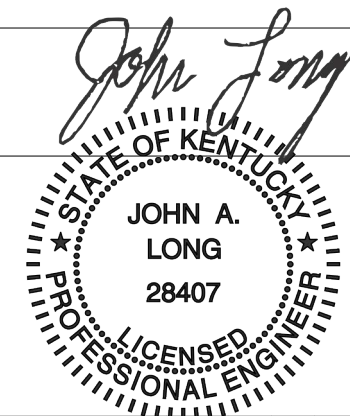
BASE PLATE TABLE			
Col Mark	Plate Size		
	Width	Thick	Length
BP-1	8"	1/2"	1'-2"

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk	x Length	W x Thk	x Length
RF2-1	12.0	32.0	0.250	177.6	8 x 1/4" x 208.3		8 x 3/8" x 178.7	
	32.0	26.2	0.313	35.0	8 x 1/4" x 35.1			
RF2-2	32.0	13.0	0.250	209.7	6 x 1/4" x 209.7		6 x 3/8" x 210.6	
	13.0	13.0	0.135	240.0	6 x 5/16" x 240.0		6 x 1/4" x 237.8	

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
 FBXXA(2)=FLANGE BRACE AT BOTH SIDES
 A - L2x2x14



RIGID FRAME ELEVATION: FRAME LINE 3



4/13/2021

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FOR PERMIT

NOTE: A325 high strength bolt shall be tightened with one washer. Refer to general notes 1.5 and 1.6 on cover sheet for tightening methods and installation inspections.

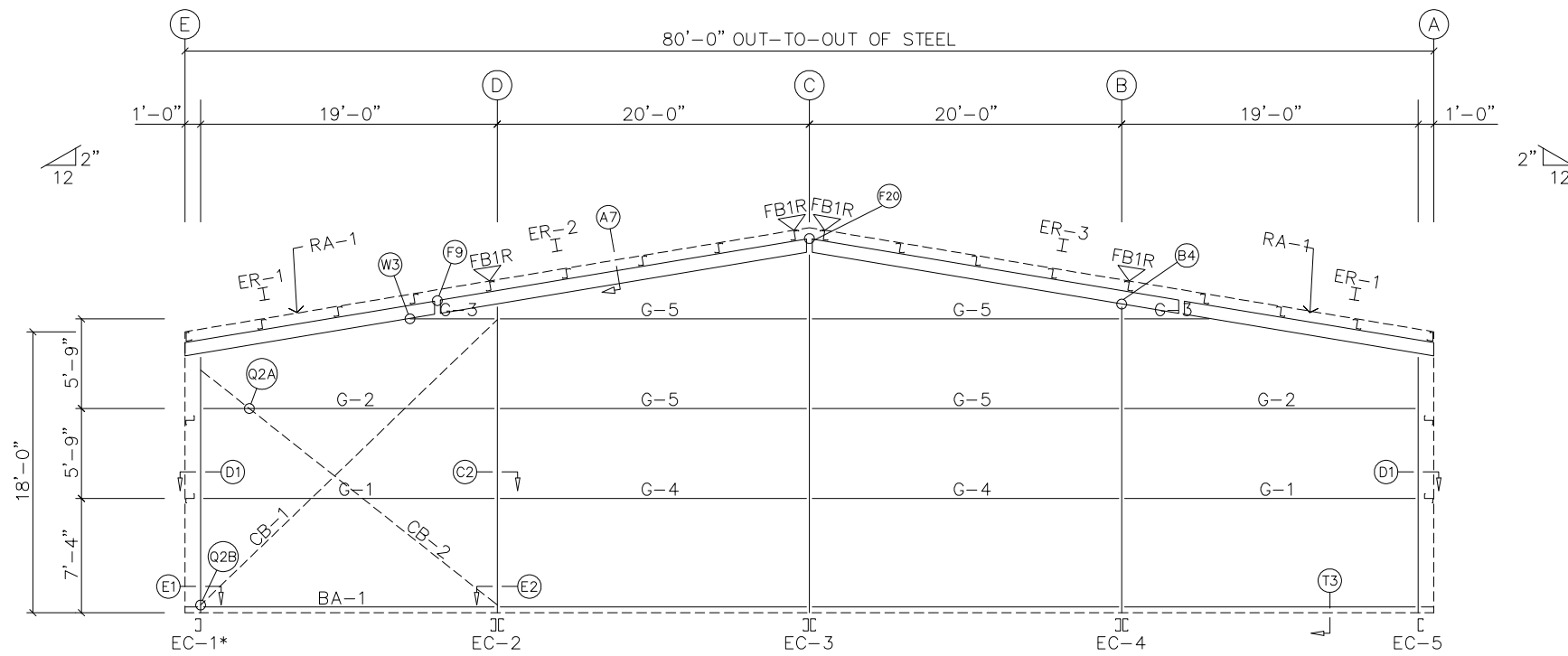
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	RIGID FRAME ELEVATION
A	PERMIT	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstown
						END USER	City of Bardstown
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstown, KY 40004
						SCALE	1:12
						SCALE	N.T.S.
						SCALE	E004
						SCALE	A



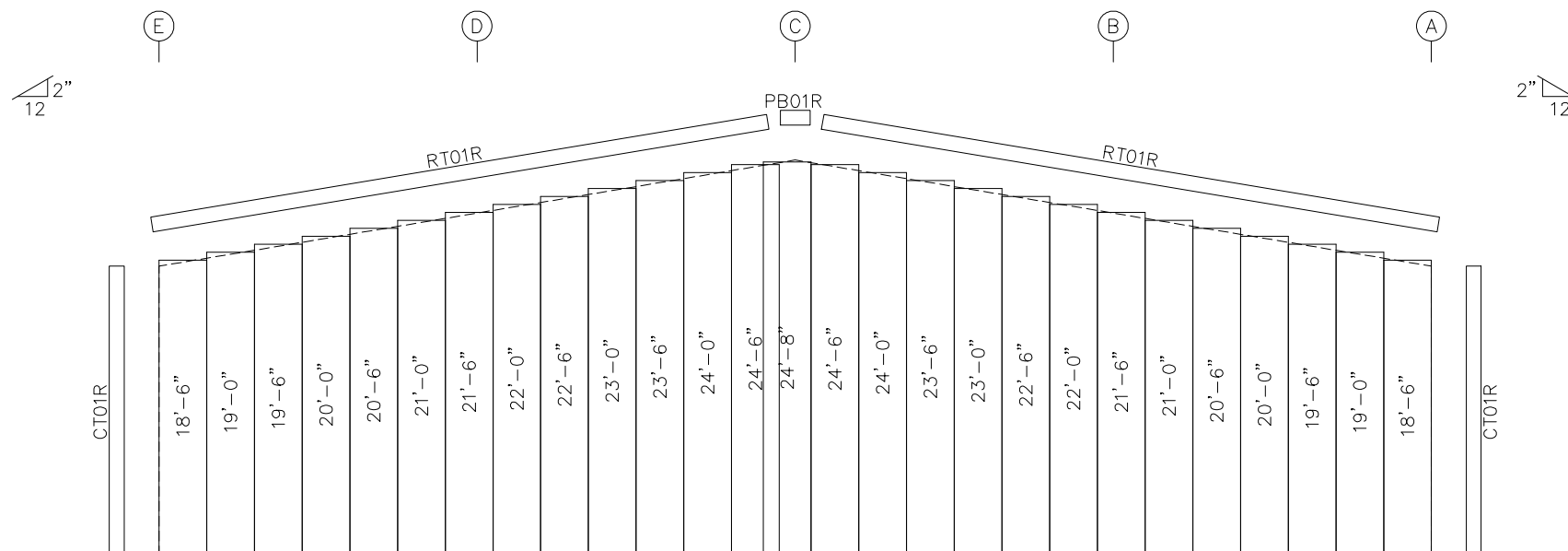
BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	1 3/4"
ER-2/ER-3	6	A325	5/8"	1 3/4"
ER-1/ER-3	8	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

FLANGE BRACE TABLE		
FRAME LINE 1		
VID	MARK	LENGTH
1	FB1R	1'-5 1/2"

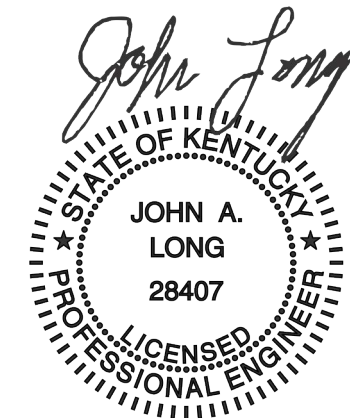
MEMBER TABLE	
FRAME LINE 1	
MARK	PART
EC-1*	10x35C14
EC-2	10x70D14
EC-3	10x70D12
EC-4	10x70D14
EC-5	10x35C14
ER-1	W8x10
ER-2	W8x10
ER-3	W8x10
G-1	8x35Z12
G-2	8x25Z12
G-3	8x25Z16
G-4	8x35Z12
G-5	8x25Z12
CB-1	CB0250
CB-2	CB0250



ENDWALL FRAMING: FRAME LINE 1
WITH 6" THK OS INSULATION (BY OTHERS)



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. PBR - Lt.Stone



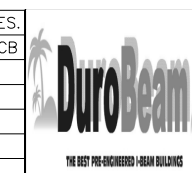
4/13/2021

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FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB

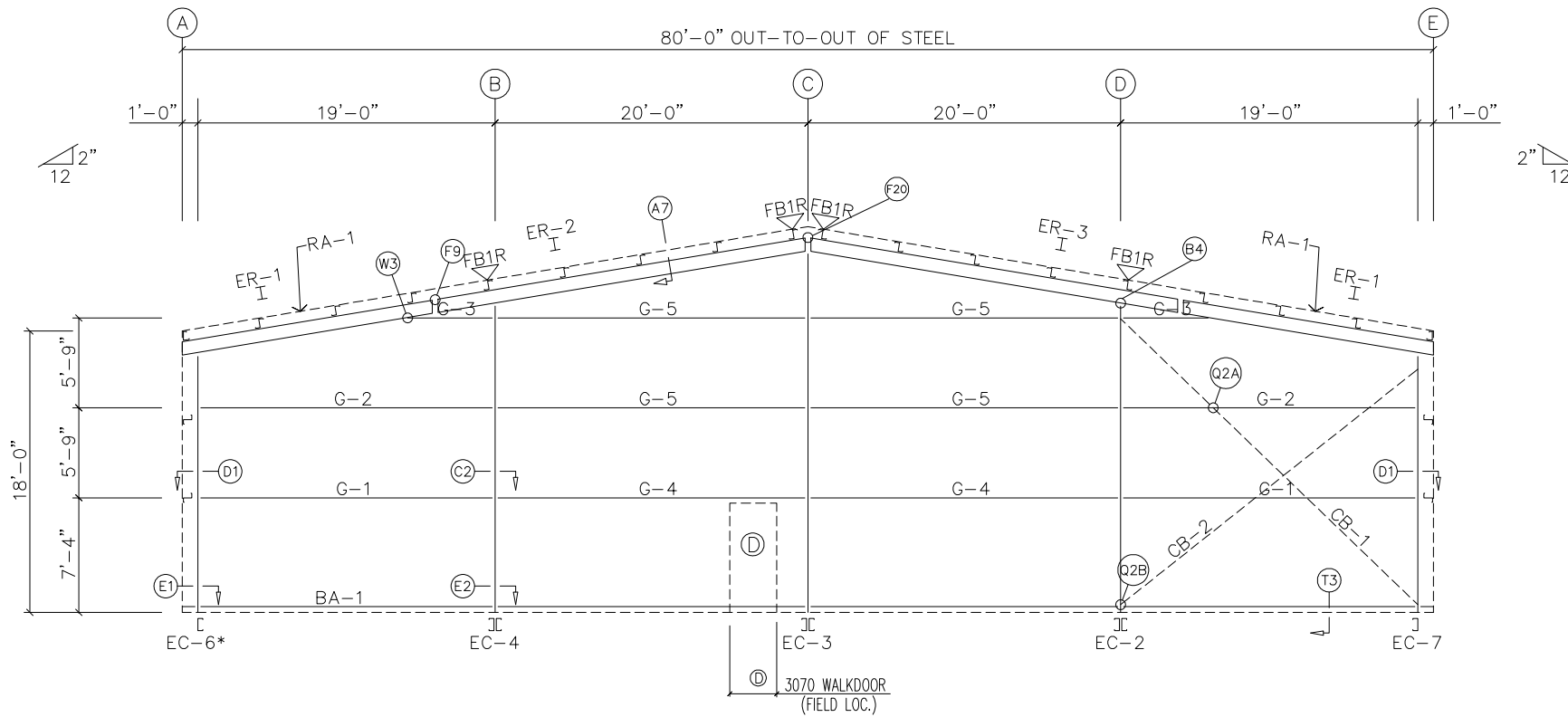
DESCRIPTION	ENDWALL FRAMING, SHEETING & TRIMS				
CUSTOMER	City of Bardstown				
END USER	City of Bardstown				
END USE	Cable Building	BUILDING	A		
STREET	999 Kelly Drive				
CITY ST ZIP	Bardstown, KY 40004				
SALES NO.	70112	JOB NO.	152536	SCALE	N.T.S.
				DATE	E005
				ISSUE	A



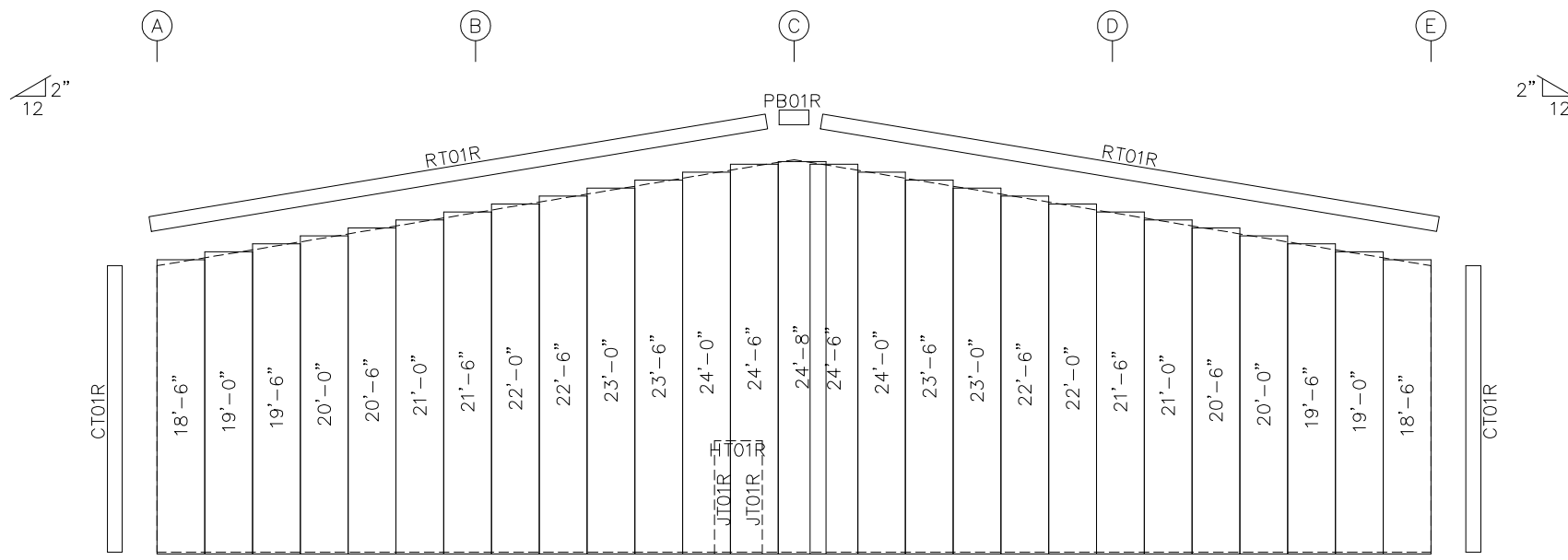
BOLT TABLE				
FRAME LINE 6				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	1 3/4"
ER-2/ER-3	6	A325	5/8"	1 3/4"
ER-1/ER-3	8	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

FLANGE BRACE TABLE		
FRAME LINE 6		
VID	MARK	LENGTH
1	FB1R	1'-5 1/2"

MEMBER TABLE	
FRAME LINE 6	
MARK	PART
EC-2	10x70D14
EC-3	10x70D12
EC-4	10x70D14
EC-6*	10x35C14
EC-7	10x35C12
ER-1	W8x10
ER-2	W8x10
ER-3	W8x10
G-1	8x35Z12
G-2	8x25Z12
G-3	8x25Z16
G-4	8x35Z12
G-5	8x25Z12
CB-1	CB0250
CB-2	CB0250



ENDWALL FRAMING: FRAME LINE 6
 (D) - 3070M WALKDOOR (FIELD LOCATE)
 WITH 6" THK OS INSULATION (BY OTHERS)



ENDWALL SHEETING & TRIM: FRAME LINE 6
 PANELS: 26 Ga. PBR - Lt.Stone



4/13/2021

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FOR PERMIT

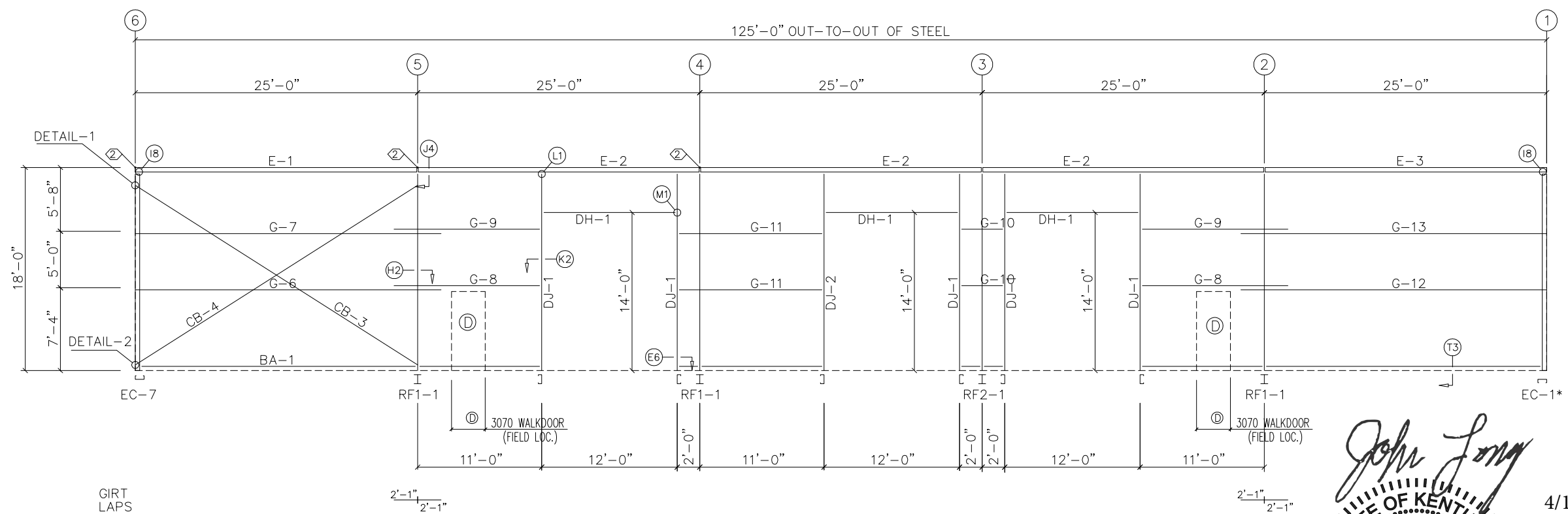
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB

DESCRIPTION	ENDWALL FRAMING ,SHEETING & TRIMS
CUSTOMER	City of Bardstown
END USER	City of Bardstown
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstown, KY 40004
SALES NO.	70112
JOB NO.	152536
SCALE	N.T.S.
DWG. NO.	E006
ISSUE	A



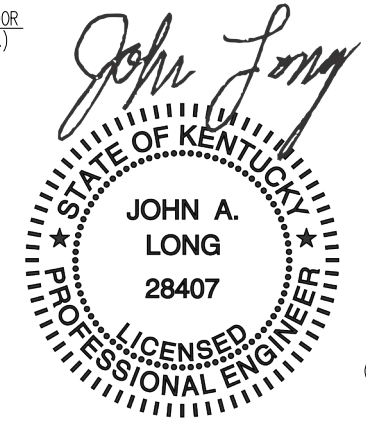
SPECIAL BOLTS					
Q ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A307	1/2"	1 1/4"	0

MEMBER TABLE	
FRAME LINE E	
MARK	PART
DJ-1	8x25C16
DJ-2	8x25C14
DH-1	8x25C16
E-1	L8ES16
E-2	L8ES16
E-3	L8ES16
G-6	8x35Z14
G-7	8x25Z14
G-8	8x35Z14
G-9	8x25Z16
G-10	8x25Z16
G-11	8x25Z16
G-12	8x35Z14
G-13	8x25Z14
CB-3	CB0500
CB-4	CB0500

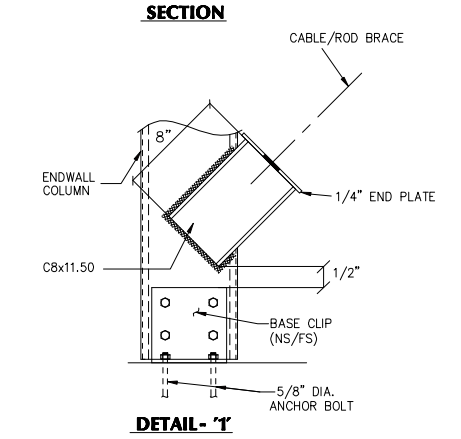
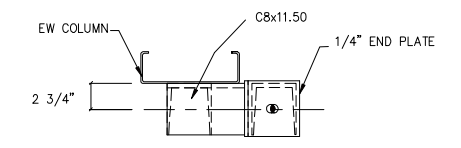
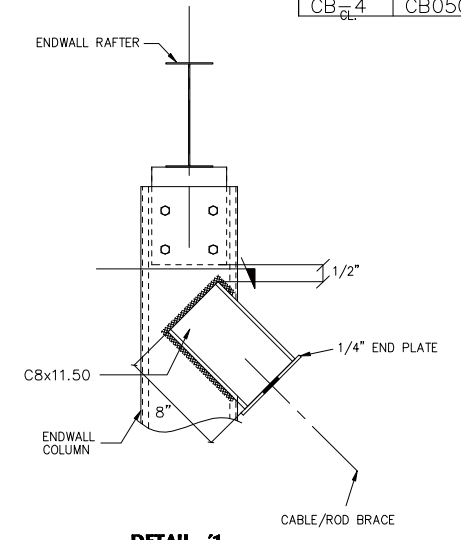


SIDEWALL FRAMING: FRAME LINE E

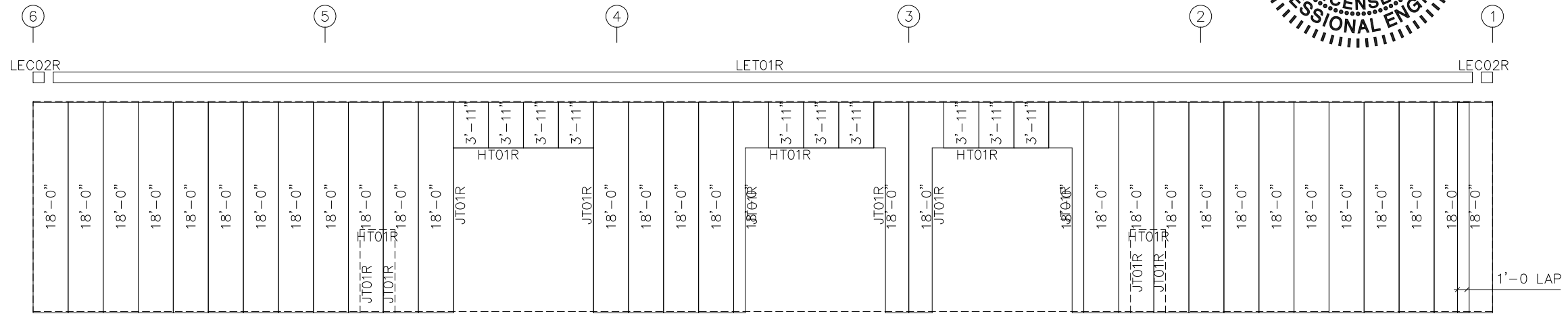
Ⓧ - 3070M WALKDOOR (FIELD LOCATE)
WITH 6" THK OS INSULATION (BY OTHERS)



4/13/2021



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SIDEWALL SHEETING & TRIM: FRAME LINE E

26 Ga. PBR - Lt.Stone

START OF PANEL →
126 NOS OF PANEL



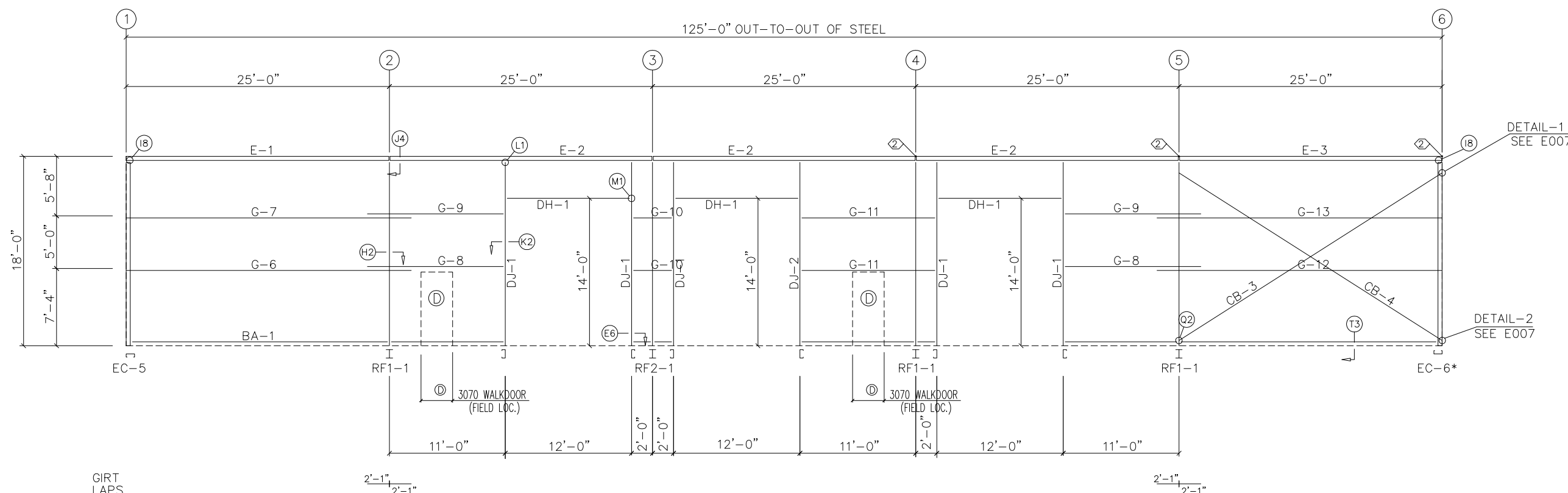
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB



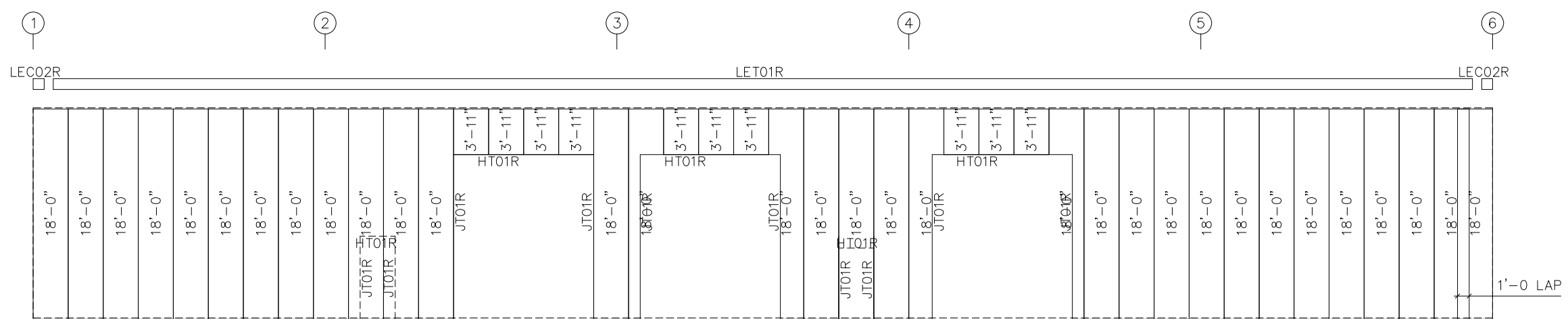
DESCRIPTION	SIDEWALL FRAMING. SHEETING & TRIMS				
CUSTOMER	City of Bardstown				
END USER	City of Bardstown				
END USE	Cable Building	BUILDING	A		
STREET	999 Kelly Drive				
CITY ST ZIP	Bardstown, KY 40004				
SCALE NO.	70112	SCALE	N.T.S.	ISSUE NO.	E007

SPECIAL BOLTS					
Q ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A307	1/2"	1 1/4"	0

MEMBER TABLE	
FRAME LINE A	
MARK	PART
DJ-1	8x25C16
DJ-2	8x25C14
DH-1	8x25C16
E-1	L8ES16
E-2	L8ES16
E-3	L8ES16
G-6	8x35Z14
G-7	8x25Z14
G-8	8x35Z14
G-9	8x25Z16
G-10	8x25Z16
G-11	8x25Z16
G-12	8x35Z14
G-13	8x25Z14
CB-3	CB0500
CB-4	CB0500



SIDEWALL FRAMING: FRAME LINE A
 (D) - 3070M WALKDOOR (FIELD LOCATE)
 WITH 6" THK OS INSULATION (BY OTHERS)



SIDEWALL SHEETING & TRIM: FRAME LINE A
 26 Ga. PBR - Lt.Stone

John Long

4/13/2021

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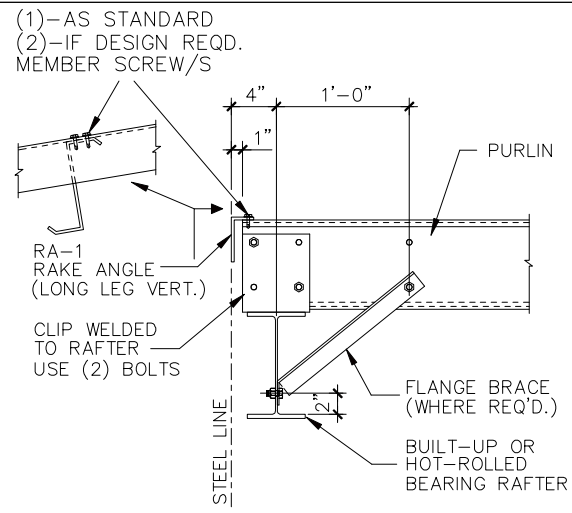
START OF PANEL →
 126 NOS OF PANEL

FOR PERMIT

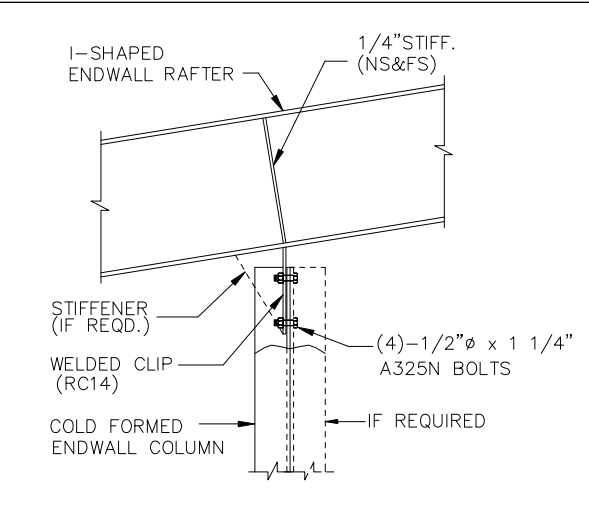
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB



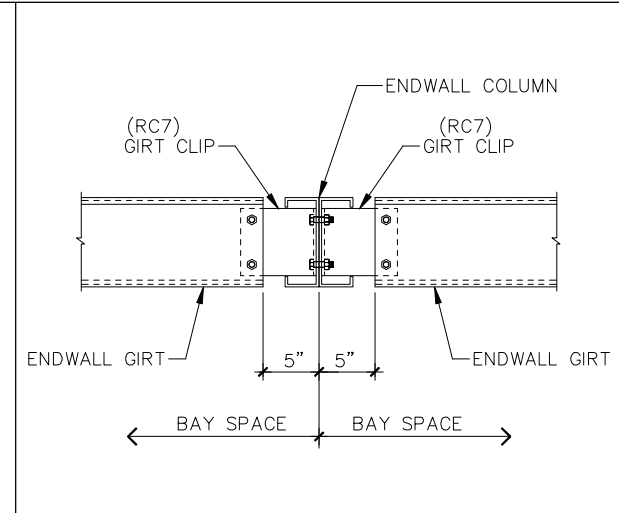
DESCRIPTION	SIDEWALL FRAMING, SHEETING & TRIMS				
CUSTOMER	City of Bardstown				
END USER	City of Bardstown				
END USE	Cable Building	BUILDING	A		
STREET	999 Kelly Drive				
CITY ST ZIP	Bardstown, KY 40004				
SALES NO.	70112	JOB NO.	152536	SCALE	N.T.S.
DATE	4/13/21	ISSUE NO.	E008	ISSUE	A



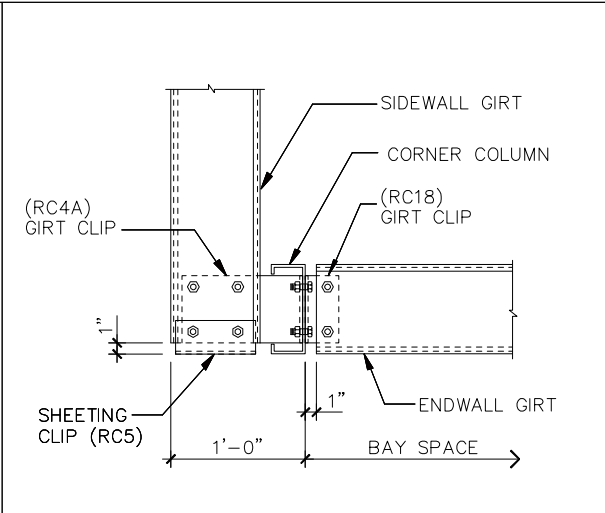
A7 BEARING FRAME TO FLUSH ENDWALL
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



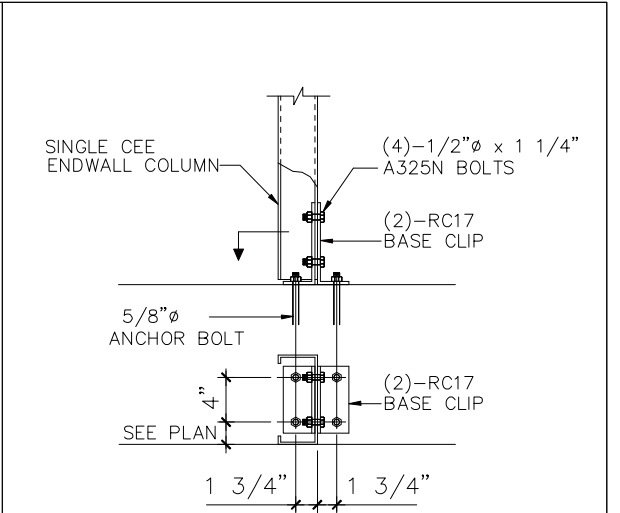
B4 ENDWALL RAFTER TO COLUMN
ALL BOLTS AS NOTED



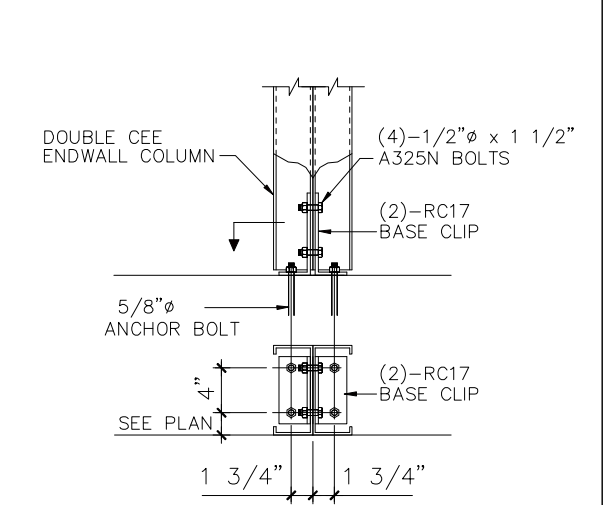
C2 ENDWALL COLUMN TO WALL GIRTS
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



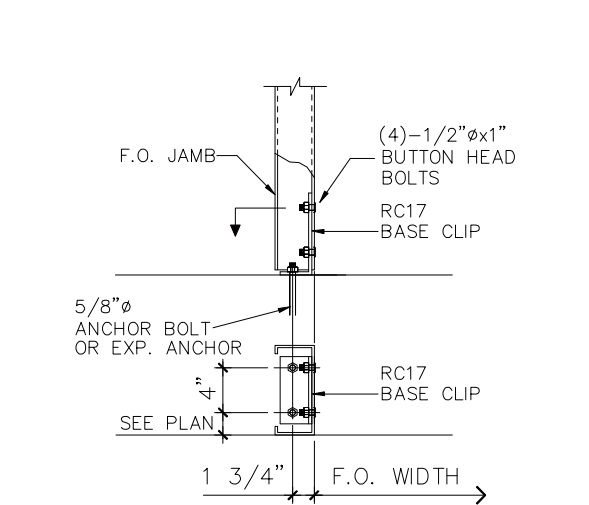
D1 SINGLE CEE CORNER COLUMN
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



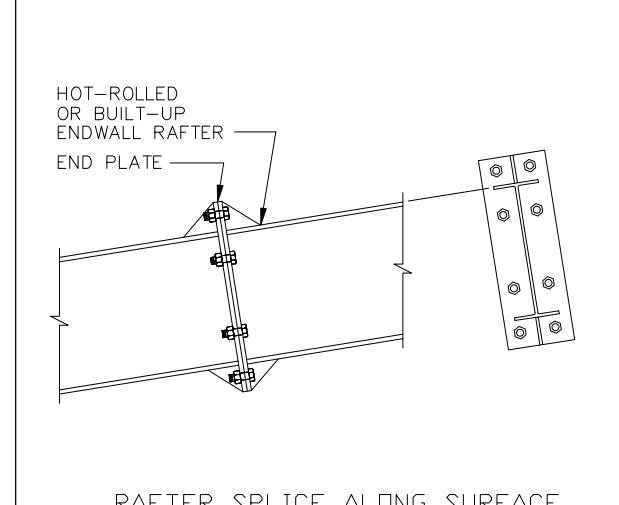
E1 ENDWALL COLUMN BASE DETAIL
ALL BOLTS AS NOTED



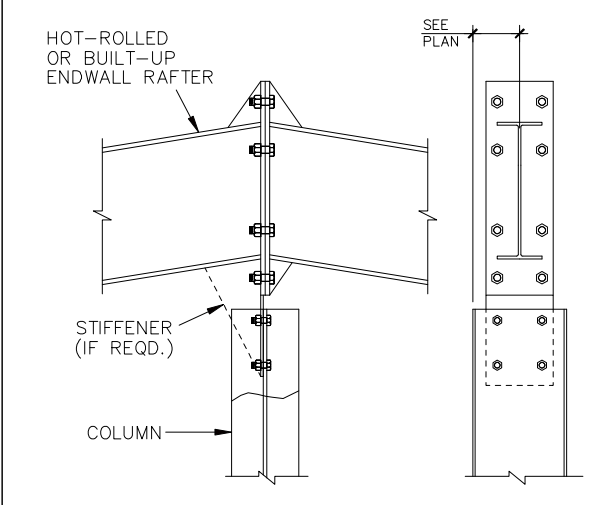
E2 ENDWALL COLUMN BASE DETAIL
ALL BOLTS AS NOTED



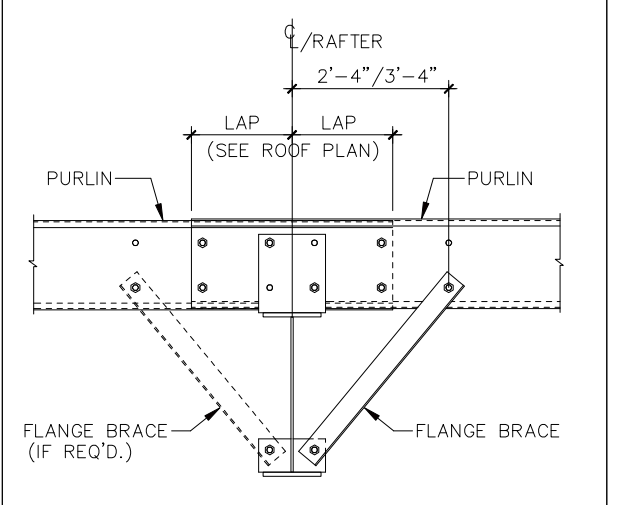
E6 "FO" JAMB BASE DETAIL WITH BOLTED BASE CLIP



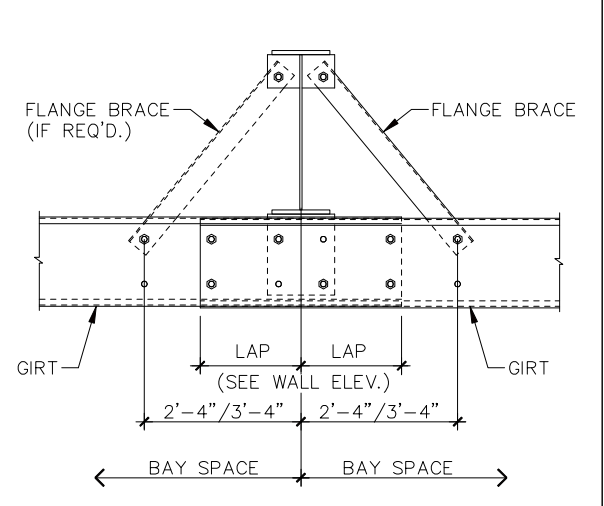
F9 RAFTER SPLICE ALONG SURFACE HOT-ROLLED OR BUILT-UP RAFTER
SEE ENDWALL FRAMING ELEV. FOR BOLT DIA AND TYPE.



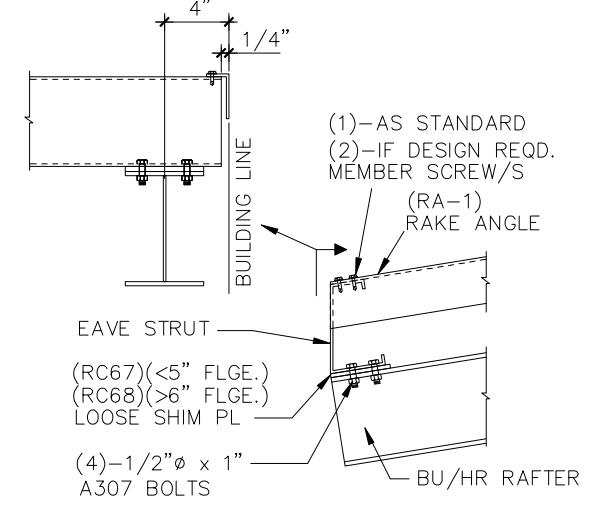
F20 RAFTER SPLICE AT RIDGE WITH COLUMN
SEE ENDWALL FRAMING ELEV. FOR BOLT DIA AND TYPE.



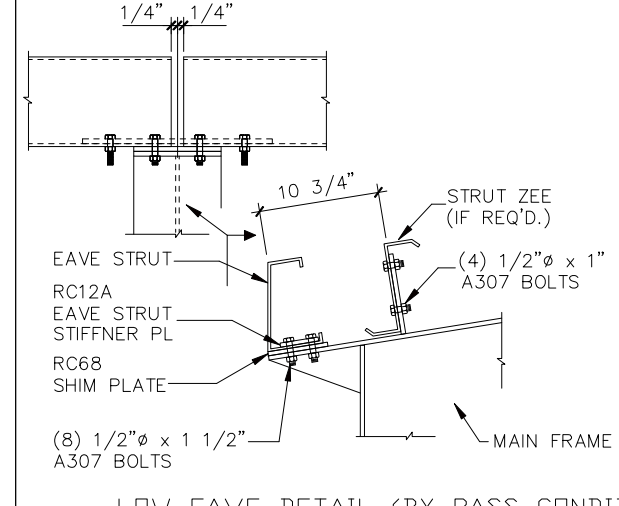
G2 BY-PASS PURLIN TO RAFTER DETAIL
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



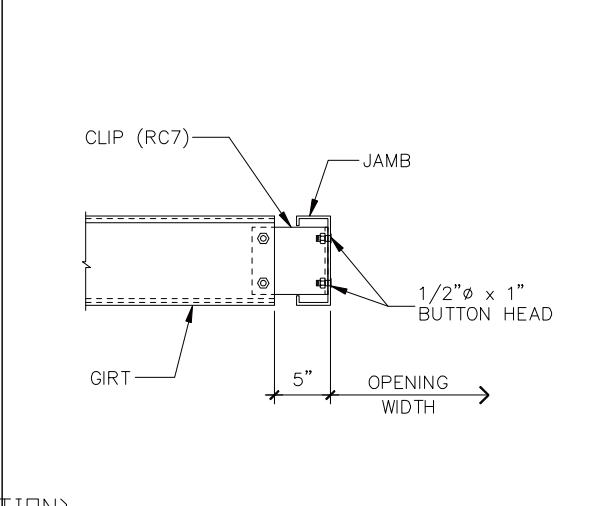
H2 WALL GIRTS TO RF COLUMN
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



I8 EAVE STRUT TO ENDWALL RAFTER LIDS



J4 LOW EAVE DETAIL (BY-PASS CONDITION) WITH EAVE STRUT STIFFENER PLATE AT INTERIOR FRAME



K2 GIRTS TO FRAMED OPENING JAMB
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.

John Long
STATE OF KENTUCKY
JOHN A. LONG
28407
LICENSED PROFESSIONAL ENGINEER

4/13/2021

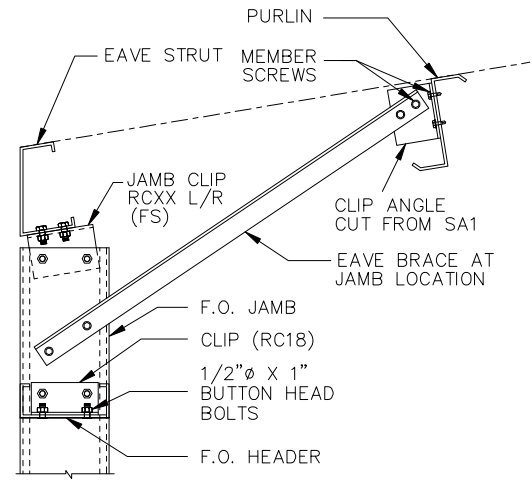
SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT DURO BEAM ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY D.B.S.B. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN D.B.S.B. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

FOR PERMIT

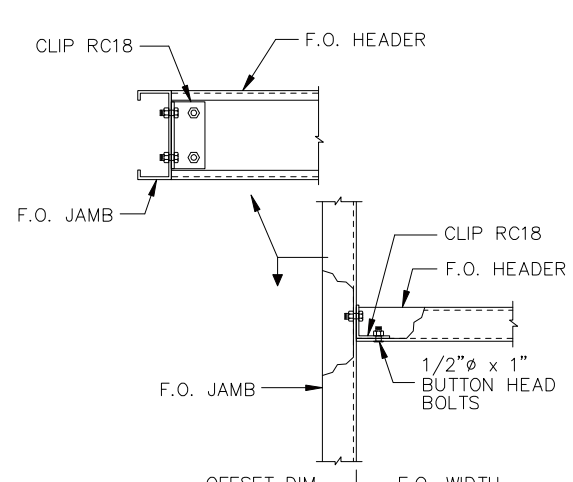
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB



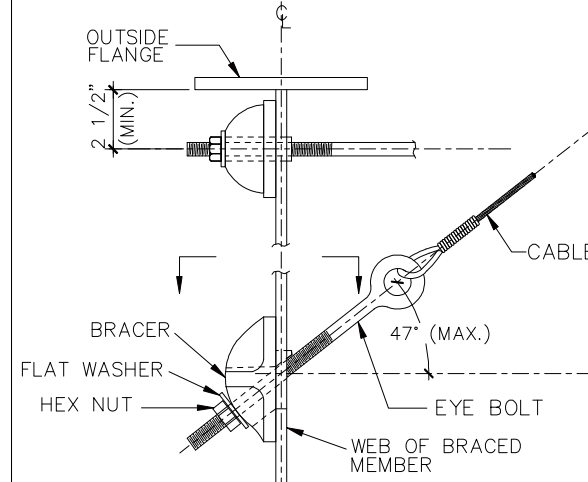
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	City of Bardstovn
END USER	City of Bardstovn
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstovn, KY 40004
SCALE NO.	70112
DR. NO.	152536
ISS. NO.	N.T.S.
CHK. NO.	E009
ISSUE	A



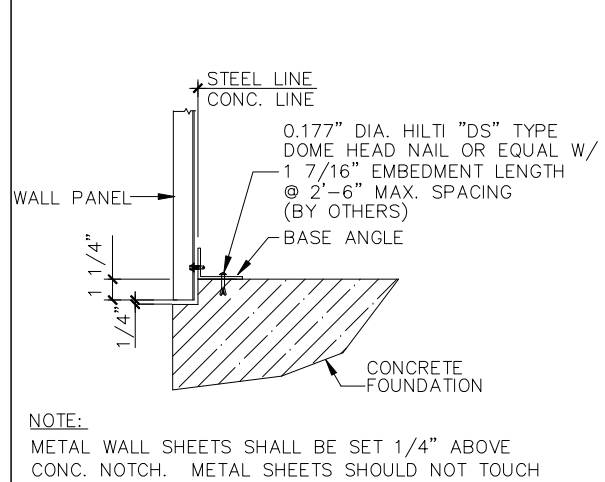
L1 F.O. JAMB TO EAVE STRUT CONNECTION FOR OVERHEAD DOOR, ROLL-UP DOOR
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



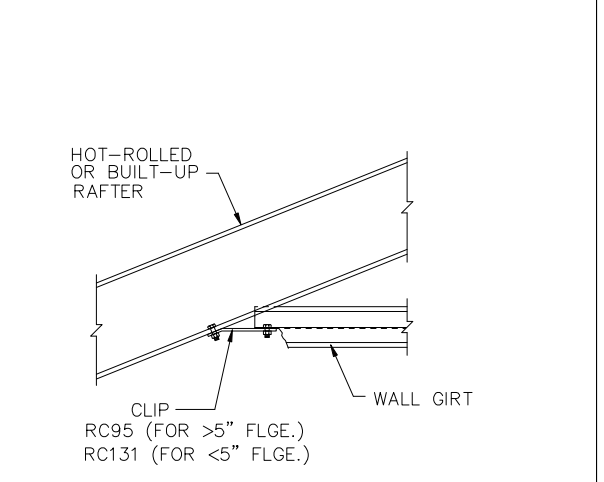
M1 F.O. HEADER TO F.O. JAMB
ALL BOLTS ARE 1/2"Ø x 1" A307 U.N.



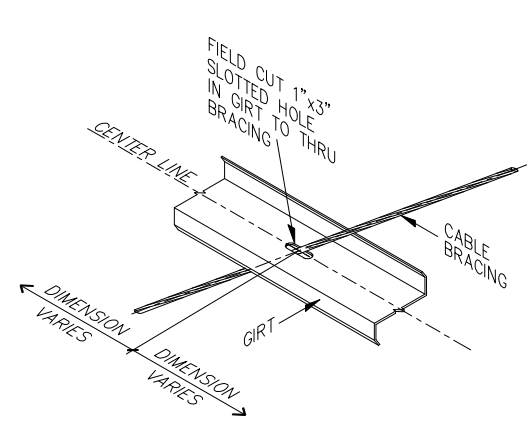
Q2 CABLE BRACE CONNECTION DETAIL



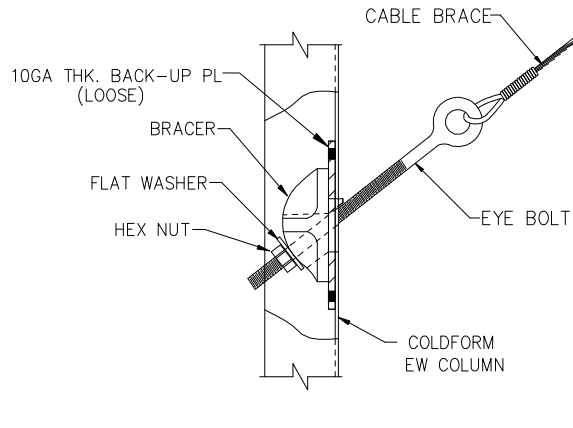
T3 DETAIL BASE w/ NOTCH
ID T3_NOTCH; NO TRIM-ANGLE-NOTCH



W3 ENDWALL GIRT TO RAFTER CONNECTION
ALL BOLTS ARE 1/2"Ø x 1" A307 BOLTS



Q2A CABLE BRACING THRU GIRT



Q2B DIAGONAL CABLE, EYEBOLT END DETAIL AT COLD-FORM COLUMN

FOR PERMIT

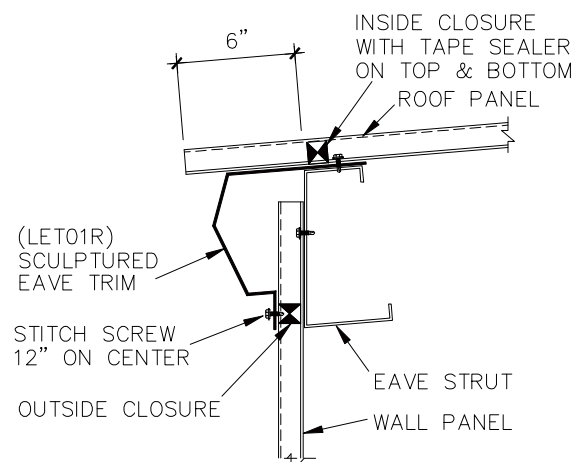


4/13/2021

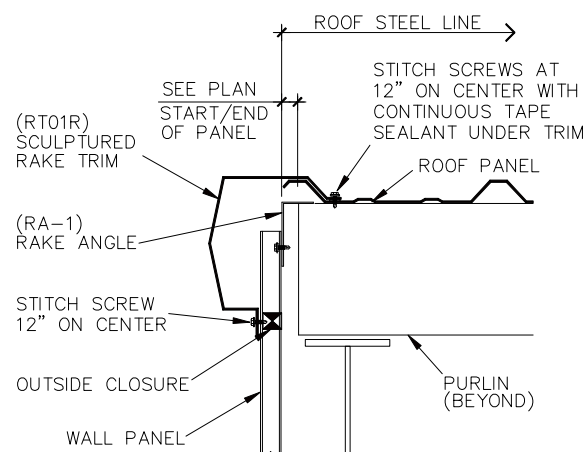
SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT DURO BEAM ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY D.B.S.B. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN D.B.S.B. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB

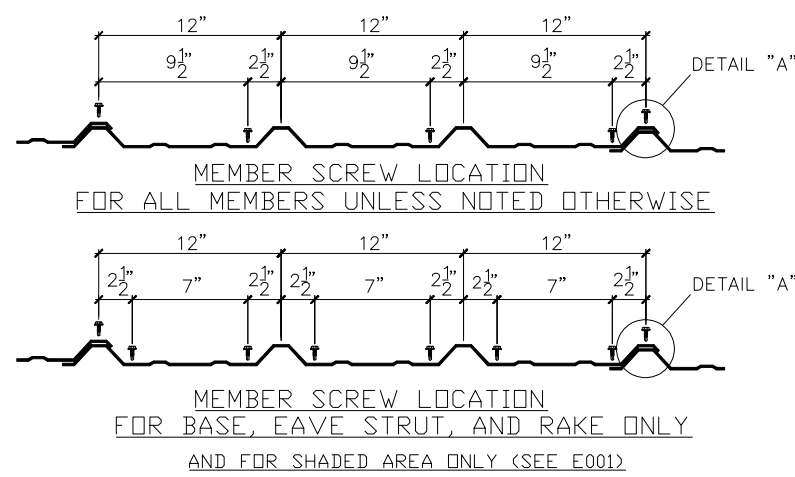
DESCRIPTION	DETAIL DRAWINGS 2			
CUSTOMER	City of Bardstown			
END USER	City of Bardstown			
END USE	Cable Building	BUILDING	A	
STREET	999 Kelly Drive			
CITY ST ZIP	Bardstown, KY 40004			
SCALE	70112	JOB NO.	152536	SCALE: N.T.S. SHEET NO. E010



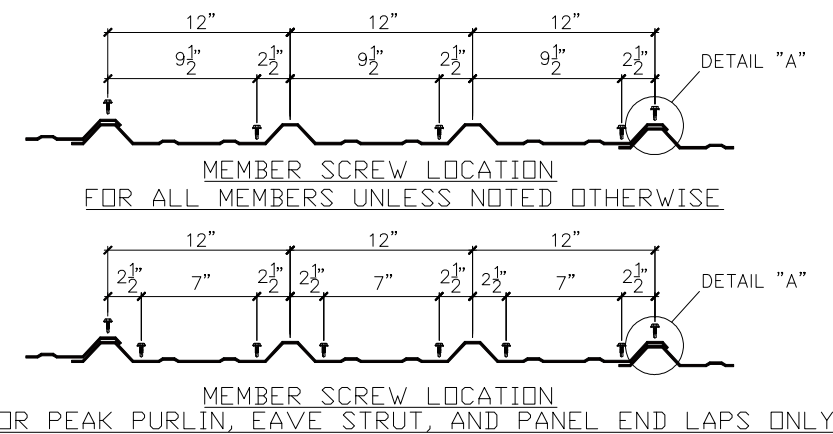
LOW EAVE DETAIL WITH SCULPTURED EAVE TRIM SHEETED WALL



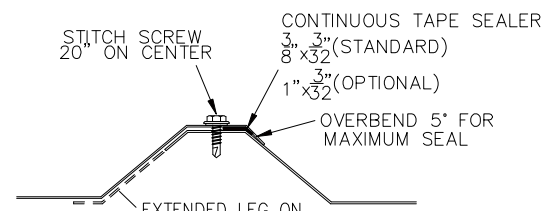
SCULPTURED RAKE DETAIL WITH SHEETED WALL "R" AND "PBR" ROOF PANEL



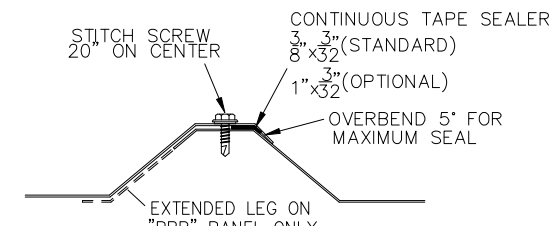
MEMBER SCREW LOCATION FOR ALL MEMBERS UNLESS NOTED OTHERWISE



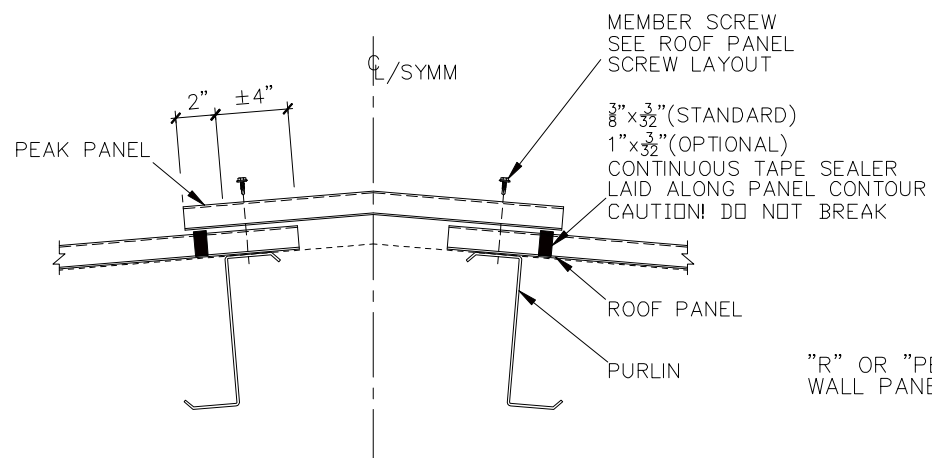
MEMBER SCREW LOCATION FOR PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS ONLY



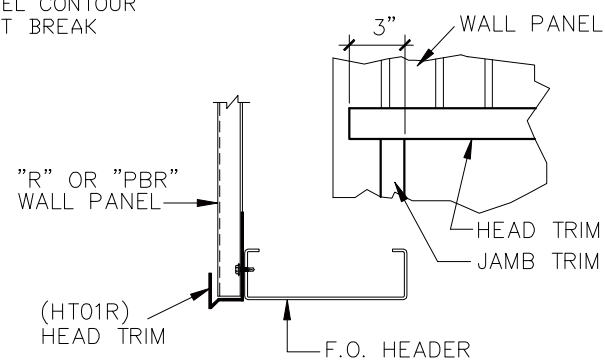
TYP. SIDE LAP DETAIL "PBR" WALL PANEL SCREW LAYOUT



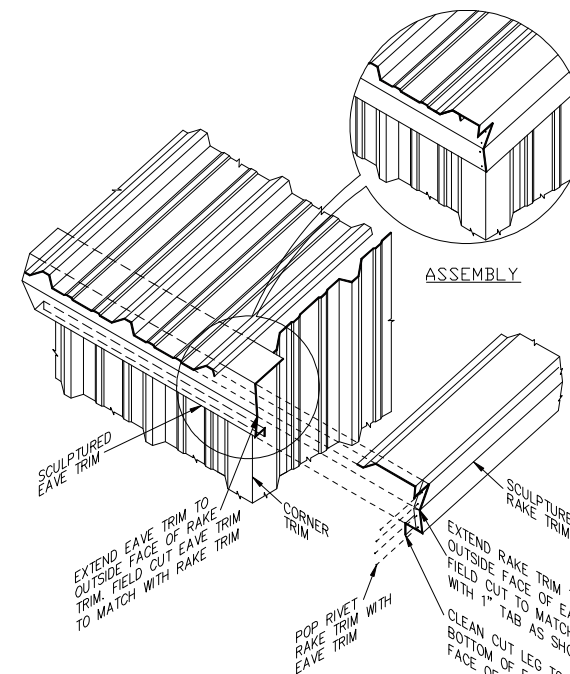
TYP. SIDE LAP DETAIL "PBR" ROOF PANEL SCREW LAYOUT



PEAK PANEL DETAIL

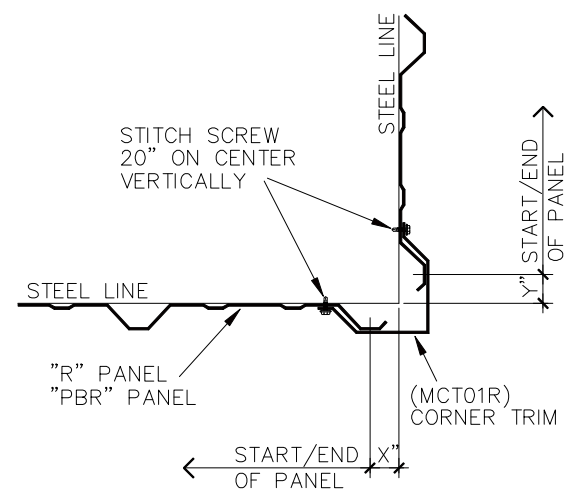


F.O. HEADER TRIM DETAIL

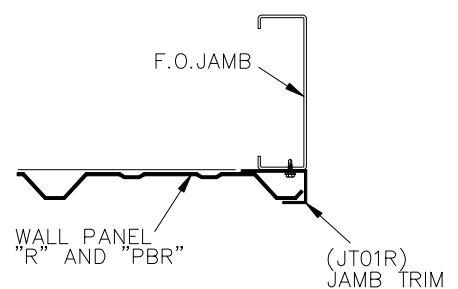


SCULPTURED EAVE & RAKE TRIM JOINT DETAIL WITH "R" AND "PBR" ROOF PANEL

FOR PERMIT



OUTSIDE CORNER DETAIL OFF MODULE



F.O. JAMB TRIM DETAIL

John Long
 STATE OF KENTUCKY
 JOHN A. LONG
 28407
 LICENSED PROFESSIONAL ENGINEER

4/13/2021

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT DURO BEAM ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY D.B.S.B. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN D.B.S.B. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	04/13/21	EPN	FLT	LCB

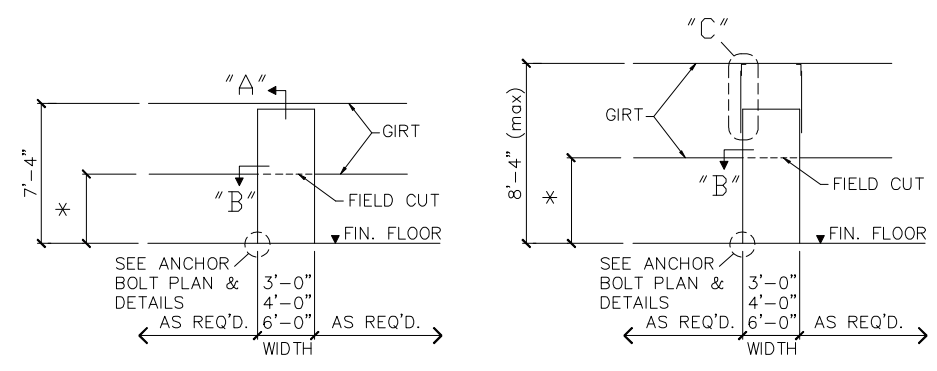
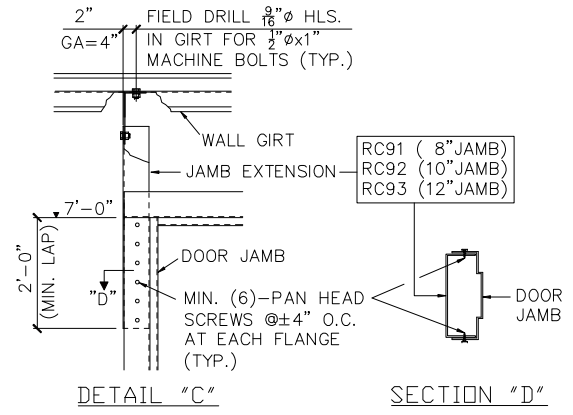
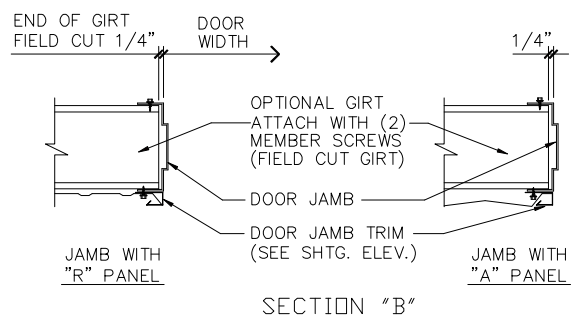
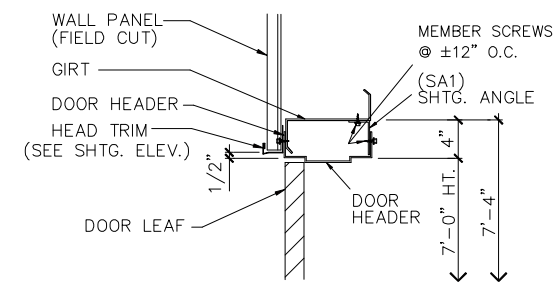
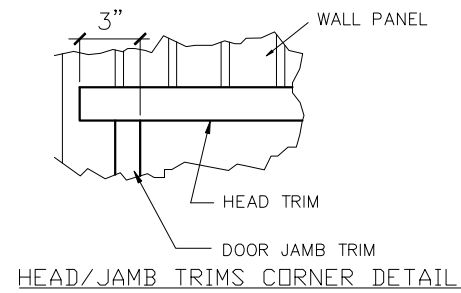
DESCRIPTION	PANEL PROFILE, TRIMS & ACCESSORIES
CUSTOMER	City of Bardstovwn
END USER	City of Bardstovwn
END USE	Cable Building BUILDING A
STREET	999 Kelly Drive
CITY ST ZIP	Bardstovwn, KY 40004
SCALE NO.	152536
SCALE	N.T.S.
DWG. NO.	E011
ISSUE	A



NOTES:
 1. FOR 3070, 4070, 6070 WALK DOORS ONLY
 2. ALL DOORS ARE FIELD LOCATED UNLESS SHOWN IN A.B. PLAN
 3. * DIMENSION VARIES. SEE WALL ELEVATION IF REQUIRED.

INSTALLATION PROCEDURE

1. Place head section and jambs on flat surface (floor) with door side up. Install bolts and nuts connecting head to jambs. Be sure that head is tight to jambs so that the proper door opening is obtained.
2. Install door leaf in frame, check for 1/8" clearance at head and 3/32" clearance at striker jamb.
3. Tilt up the entire assembly and anchor hinge jamb to floor. Plumb hinge jamb and assembly. Field cut girts if required.
4. Anchor head and striker jamb to building structure, floor and entire frame to panel skins (field cut). Install optional threshold anchor if desired. Install jamb extensions (if req'd).
5. Install lockset. Install (optional) weatherstrip, head member first. Adjust so that vinyl just contacts door when in the closed position. Do not force vinyl against door as this will interfere with the latching and will not improve the weather seal.
6. Refer also to Door Manufacturer Installation Manual for more details.



FOR PERMIT

John Long
 STATE OF KENTUCKY
 JOHN A. LONG
 28407
 LICENSED ENGINEER
 PROFESSIONAL ENGINEER

4/13/2021

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.	DESCRIPTION	ACCESSORIES
A	PERMIT	04/13/21	EPN	FLT	LCB	CUSTOMER	City of Bardstown
						END USER	City of Bardstown
						END USE	Cable Building BUILDING A
						STREET	999 Kelly Drive
						CITY ST ZIP	Bardstown, KY 40004
						SALES NO.	70112
						JOB NO.	152536
						SCALE	N.T.S.
						DWG. NO.	E012
						ISSUE	A



STRUCTURAL GENERAL NOTES — APPLICABLE TO ALL CONSTRUCTION UNLESS OTHERWISE NOTED ON THE PLANS

A. DESIGN SCOPE BY PSE CONSULTING ENGINEERS (PSE)

- 1. Design Shown on drawings by PSE is for the following items.
a. Foundation for pre-engineered metal building.
2. Design Shown on PSE drawings does not include: finishes, architectural items, windows, doors, moisture barriers, water proofing, mechanical units, plumbing, or electrical items.

B. GENERAL REQUIREMENT:

- 1. Scope of service provided by PSE is limited to foundation design according to the loads given by the metal building manufacturer dated 04/13/2021.
2. Steel columns, posts, walls etc. shown on PSE drawings are schematic only. Verify actual size locations and details from drawing provided by DuroBeam Steel Buildings, the metal building manufacturer.
3. Furnish all labor, materials, and equipment necessary to complete the work shown or inferred by these drawings.
4. Where construction details are not shown or noted for any part of the work, such details shall be the same as for similar work shown on the drawings.
5. Notes and details on the drawings take precedence over the general notes and typical details in case of conflict.
6. Provide manufacturer's approved product evaluation reports (ICC reports) and a list of all proposed substitutions to the Engineer for review and written approval before fabrication.
7. Pipes, ducts, sleeves, chases, etc. shall not be placed in slabs, beams, or walls unless specifically shown or noted nor shall any structural member be cut for pipe, ducts, etc., unless specifically shown. Obtain prior written approval for installation of any additional holes, ducts, etc.
8. Locate and protect underground or concealed conduit, plumbing or other utilities where new work is being performed.
9. The contract drawings and specifications represent the finished structure and do not indicate methods, procedures or sequence of construction. The contractor shall take necessary precautions to maintain and insure the integrity of the new and any existing structures during construction. The design stresses shall not be exceeded during construction based on the age of each element. Neither the owner nor Architect/Engineer will enforce safety measure regulations. Contractor shall design, construct and maintain all safety devices, including shoring and bracing for the new and any existing structures and shall be solely responsible for conforming to all local, state and federal safety and health standards, laws and regulations.
10. Obtain prior written approval for any changes to the drawings.
11. The contractor shall review and compare the PSE structural drawings with all other Construction Documents, such as metal building manufacture drawings, Architectural, Mechanical and Electrical drawings, specifications, etc... The contractor shall verify dimensions, elevations and all information. Report, in writing, any inconsistencies, errors, or omissions to the Architect/Engineer of record before proceeding with the work.
12. All existing constructions, if any, are shown schematic only. Contractor is responsible to verify actual conditions and allow for them in his bid. Notify the Architect/Engineer, in writing, in case of any discrepancy between actual conditions and what is shown on the structural drawings before proceeding with the work.
13. See Architectural, Mechanical, Electrical and other drawings for embedded items.
14. Submit structural drawings signed and sealed by a professional Engineer licensed in the State where the project is located for any structural member needed for this project that is not designed by the Engineer of Record.
15. Any substitutions for structural members, hardware or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
16. All communications shall be in writing no verbal communication shall be validated.

C. CODE AND LOADS:

- 1. All design, material, and construction work for this project shall conform to the 2018 Kentucky Building Code based on 2015 International Building Code (IBC).
2. Design parameters.
a. Floor Live Load = 125 psf.
b. Floor Dead Load = 15 psf.
c. Roof Live Load = 20 psf.
d. Roof dead load = 3 psf.
e. Ground Snow Load, Pg = 20 psf.
f. Flat Roof snow load = 20 psf.
g. Snow Exposure Factor, Ce = 1.0
h. Snow Load Importance Factor, I = 1.0
i. Thermal Factor, Ct = 1.0
j. Ultimate Wind Speed (3 second gust) = 115 mph
k. Wind Importance Factor, Iw = 1.0
l. Wind Exposure = C
m. Internal Pressure Coefficient = +/- 0.18
n. Components and Cladding studs = 16.44 psf
o. Seismic Importance Factor, Ie = 1.0
p. Site Class = D
q. Ss = 0.194
r. S1 = 0.104
s. Sms = 0.310
t. Sm1 = 0.247
u. Sds = 0.207
v. Sd1 = 0.165
w. Seismic Design Category = C
x. Basic Seismic Force Resisting System = Steel system not specifically detailed for seismic resistance
y. Design Base Shear = 0.069 * W
z. Approximate Fundamental Period, T = 0.133
aa. Response Modification Factor, R = 3
bb. Analysis Procedure Used = Equivalent Lateral Force Procedure

D. TESTING:

- The owner/contractor shall retain an independent testing laboratory to test the quality of:
a. Soil or fill material supporting footings and slab-on-grade.
b. Concrete.
c. All other material used in this project as required by the Engineer.
d. A copy of test results shall be sent to the Engineer of Record.

E. INSPECTION:

- The owner shall employ one or more qualified Inspectors to provide inspections during construction in accordance with the above Code. The Inspector shall be certified by the building official to perform the type of inspection specified. Inspection shall be provided for:
a. Foundation excavation.
b. Reinforcement placement, prior to closing the forms and delivery of concrete.
c. Concrete placement.
d. Bolts installed in concrete and masonry, prior to and during the placement of concrete around bolts.
e. Epoxy filled anchor bolts.
f. Structural Steel.
g. Field welding.
h. High-strength bolting.
i. During preparation and taking of test specimens.
j. See other sections of these notes for more required inspections.
Note: All discrepancies shall be brought to the immediate attention of the contractor for correction; then if not corrected, to the building official and to the Engineer in writing. The inspector shall furnish an inspection report to the building official and to the Engineer/Architect of Record.

F. FOUNDATION:

- 1. A foundation investigation and report was prepared by Greenbaum Associates Inc. 994 Longfield Ave. Louisville, KY 40215 Phone (502) 361-8447, dated 03/14/2021. The contractor shall read and follow the recommendations in this report. Contractor shall keep a copy of this report on site at all times during construction.
2. Soft soil or fill material shall be removed and replaced with competent granular engineering fill or lean concrete. The new fill shall be compacted in 8" layers to gain 98% of its maximum dry density according to ASTM D-698 standard proctor, and be capable of supporting the above bearing capacity.
3. Footing shall be stepped as required to maintain minimum required frost depth, below finished grade.
4. Use light weight equipment to compact the soil within 2 feet around foundation/basement wall.
5. Excavation shall be properly back filled Back fill for walls shall be pervious material. Do not place back fill behind walls before they have attained their design strength. Shore and protect walls from lateral loads until the supporting members are in place and have developed specified strength.
6. When the finished crawl space elevation is lower than the outside finished grade, or when it is required by the Geotechnical investigative report or building department, provide 4 inch diam. perforated drain pipe below the top of the footing. Encase the pipe in 18x18 inches free-drain crushed stone and fabric at the perimeter of the crushed stone.
7. Roof and area drainage shall be directed away from the foundation.

G. CONCRETE:

- 1. All concrete work shall conform to the American Concrete Institute's Standard Building Code Requirements for Structural Concrete, ACI 318, latest edition used by the local building department
2. MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF CONCRETE:

Table with 2 columns: TYPE OR LOCATION OF CONCRETE and MINIMUM SPECIFIED COMPRESSIVE STRENGTH (F'c). Rows include: BASEMENT WALLS, FOUNDATION AND OTHER CONCRETE NOT EXPOSED TO THE WEATHER (3,500 PSI), BASEMENT SLAB AND INTERIOR SLABS ON GRADE, EXCEPT GARAGE FLOOR SLABS (3,500 PSI), BASEMENT WALLS, FOUNDATION WALLS, EXTERIOR WALLS AND OTHER VERTICAL CONCRETE WORK EXPOSED TO WEATHER (3,500 PSI), PORCHES CARPORT SLABS AND STEPS EXPOSED TO THE WEATHER, AND GARAGE FLOOR SLABS (3,500 PSI).

- 3. Foundation is designed for 2,500 psi however, concrete strength of 3,500 psi is required for durability purpose. No special inspection is required.
4. Basement wall, foundation wall, basement slab, slab on grade, all concrete work exposed to weather, and all exterior concrete shall contain the proper admixtures to obtain 5% to 7% Air Entrainment.
5. Reinforcing Steel:
a. All reinforcing steel shall be ASTM A615 Grade 60.
b. Vertical bars shall be doweled to supporting members with the same size and spacing of reinforcement shown in the drawing or general notes.
c. Splices shall be 48 bar diameters or 30 inches whichever is greater UON.
d. All reinforcing bars shall be in the correct place, tied and secured prior to concrete placement. Use chairs, spacers and sand plates as required.
e. All concrete is reinforced concrete unless specifically called out as "Unreinforced". Reinforce all concrete not otherwise shown with same steel as in similar sections or areas.
6. All concrete shall be consolidated with mechanical vibrators.
7. All concrete work shall be cured and maintained above 50 degrees Fahrenheit for at least seven days according to the Standard Practice for Curing Concrete, ACI 308, ACI 318 and as approved by the Engineer.
8. When air temperature is above 80 degrees Fahrenheit, Hot Weather Concrete, ACI 305R shall apply. When the average air temperature is below 40 degrees Fahrenheit, Cold Weather Concrete, ACI 306R shall apply.

H. SLAB-ON-GRADE:

- 1. See "Concrete" and "Foundation notes above for additional requirements.
2. Concrete mix:
a. Slump shall not be more than 3 inches.
b. Nominal maximum size of aggregate shall be 1 inch.
3. Prior to placing concrete, prepare and compact the sub-grade and sub-base per contract document.
4. If vapor barrier is not used, dampen the sub-grade/sub-base by spraying water before concreting.
5. Finishing Slabs:
a. Do not directly apply water to slab surface or dust with cement.
b. All methods, techniques and equipment shall be as recommended in ACI 302.1R.
6. Slab surface tolerances:
a. Achieve flat, level planes except where grades are indicated. Slope uniformly to drains.
b. Troweled finishes: Achieve level surface plane so that depressions between high spots do not exceed 1/4 inch, using a 10-foot straightedge.
7. Provide sawn floor slab control joint where shown on plans. Where not shown, limit maximum spacing between control joints to 15 feet for indoor slabs, 8 feet for outdoor driveway and 5 feet for sidewalks.
8. Saw cuts shall be made immediately after final finishing without dislodging aggregate.
9. Joint fill for control joint:
a. Use elastomeric sealant for areas where no vehicle traffic exists.
b. Use semi-rigid epoxy where vehicle traffic exists.
10. At expansion joint, use premolded fiber joint material, then use joint filler as described above.

I. STRUCTURAL STEEL:

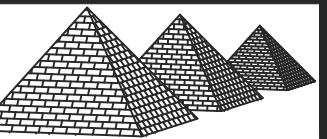
- 1. All structural steel shall be ASTM A-36, for items not provided by the building manufacturer.

J. CONTROL AND EXPANSION JOINTS:

- 1. All joints in walls and slab-on-grade shall be located as shown on drawing or as approved by the Architect/Engineer. Sealant shall be installed in accordance with specifications and approved by the Architect/Engineer.

K. ABBREVIATIONS:

Table of abbreviations: AB ANCHOR BOLT, ALT ALTERNATE, ARCH ARCHITECTURAL, BOF BOTTOM OF FOOTING, CJ CONSTRUCTION JOINT OR CONTROL JOINT, CL CENTER LINE, CLR CLEAR, CONT CONTINUOUS, DIM DIMENSION, DL DEAD LOAD, DWG DRAWING, E EXISTING, EMBED EMBEDMENT, EOR ENGINEER OF RECORD, FD FROST DEPTH, GC GENERAL CONTRACTOR, HORIZ HORIZONTAL, ICC INTERNATIONAL CODE COUNCIL, LL LIVE LOAD, MAX MAXIMUM, MFR MANUFACTURER, MIN MINIMUM, NTS NOT TO SCALE, OC ON CENTER, OH OPPOSITE HAND, OSV ON SITE VERIFY, PERP PERPENDICULAR, PLF POUND PER LINEAR FOOT, REF REFERENCE, REINF REINFORCEMENT, SIM SIMILAR, SN SHEAR NAIL, SL SNOW LOAD, SPEC SPECIFICATION, STD STANDARD, TYP TYPICAL, UON UNLESS OTHERWISE NOTED, W/ WITH, W/O WITHOUT.



PSE Consulting Engineers, Inc.

www.structure1.com
Klamath Falls Office
250 Main
Klamath Falls, Oregon 97601
Phone: (541) 850-6300
Fax: (541) 850-6233
info@structure1.com

Medford Office
836 Mason Way
Medford OR 97501
Phone: (541) 776-8500
infomd@structure1.com

Licensed in 48 States!

Construction Types:
Light Gauge Steel, Straw Bales, Bamboo, Log, Timber/Wood, Structural Insulated Panels/SIPs, Masonry, Steel, Concrete, Modular Homes/Factory Built Housing (FBI), ICF, Shipping Containers, and many more! Commercial or Residential. And Green/Sustainable!

Project:
80' x 125' Cable Building

999 Kelly Drive
Bardstown, KY 40004

Owner / Client:

DuroBeam
Job # 152536



Table with columns: MARK, REVISION SCHEDULE, DESCRIPTION, DATE. Includes a grid for tracking revisions.

DRAWN BY: R.N.

DS. BY: R.N.

CHK BY: N.T.

DATE: 06-03-2021

TITLE:

GENERAL NOTES

PAGE NO:

S1

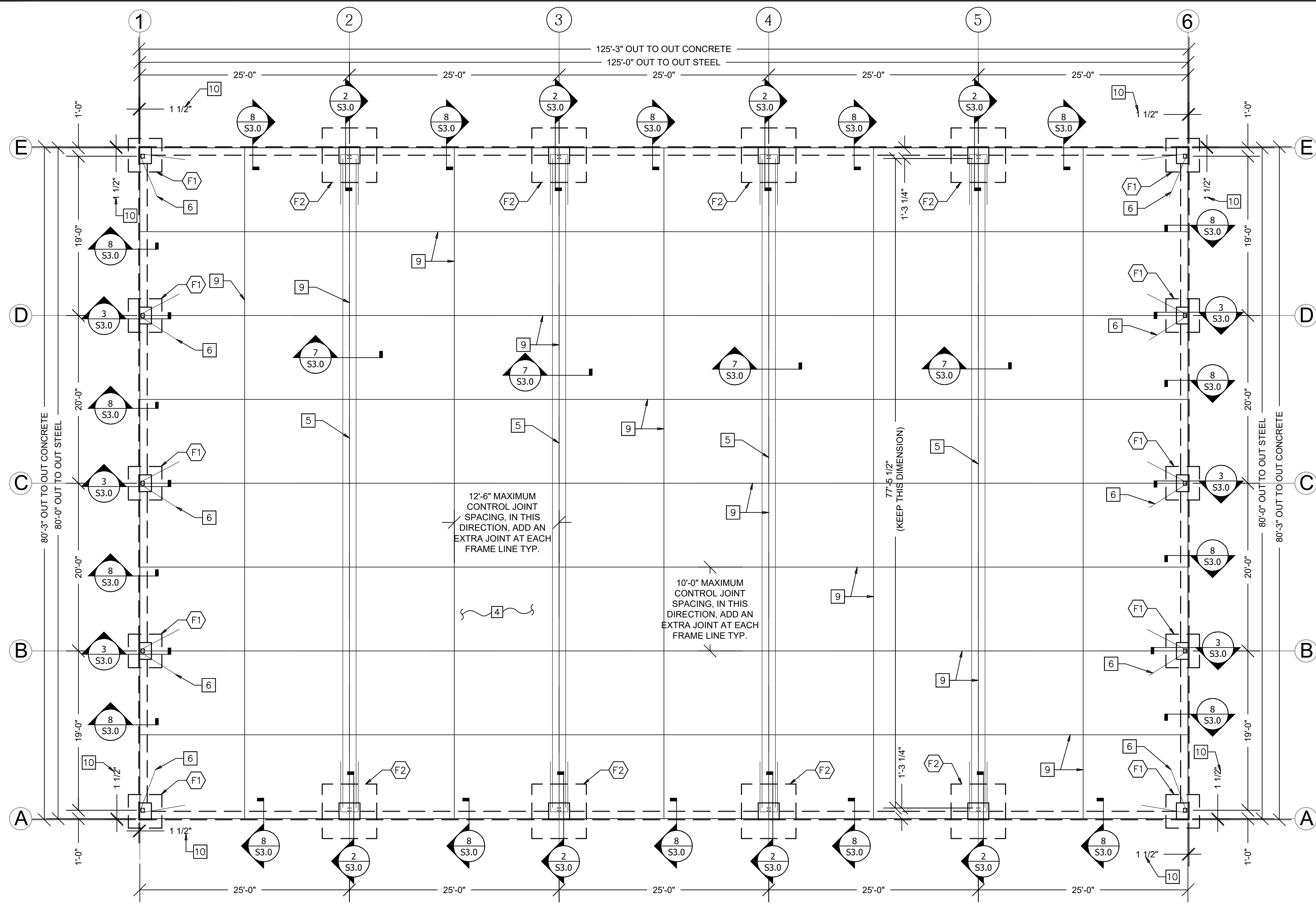
PROJECT #:
DUROBEAM
221-7

SHEET INDEX table: S1 GENERAL STRUCTURAL NOTES, S2 FOUNDATION PLAN, S3 FOUNDATION DETAILS

Structural details for this project are for illustration only. They are not drawn to scale unless noted otherwise. Contractor must verify all dimensions before fabrication or construction. Do not scale drawings.

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2021-06-03 Duro Beam 221-7-80x125 KY.dwg
6/3/2021 6:20 PM
Ridhik.Naik



1 FOUNDATION PLAN
 Scale: 1/8" = 1'-0"
 Do not scale

THE DRAWINGS SHOWN ARE FOR ONE BUILDING ONLY. CONTRACTOR/OWNER SHALL OBTAIN WRITTEN APPROVAL FROM THE ENGINEER OF RECORD FOR ANY ADDITIONAL BUILDINGS.

MARK	FOOTING			PEDESTAL FOOTING				REINFORCEMENT				DETAIL NO./REMARKS	
	DIMENSION		THICKNESS "T"	BOTTOM REINFORCEMENT		TOP REINFORCEMENT		DIMENSION		REINFORCEMENT			
	LENGTH "A"	WIDTH "B"		LONGITUDINAL	TRANSVERSE	LONGITUDINAL	TRANSVERSE	LENGTH a	WIDTH b	DEPTH c	VERTICAL		STIRRUPS
F1	6'-6"	6'-6"	1'-0"	#5 @ 12" O.C.	#5 @ 12" O.C.	#5 @ 16" O.C.	#5 @ 16" O.C.	2'-6"	2'-0"	2'-0"	(12) - #6	#4 @ 9" O.C. W/ 3 #4 @ 3" O.C. @ TOP OF PEDESTAL.	1/S3.0
F2	4'-0"	4'-0"	1'-0"	#5 @ 12" O.C.	#5 @ 12" O.C.	#5 @ 16" O.C.	#5 @ 16" O.C.	2'-0"	1'-6"	2'-0"	(8) - #6		2/S3.0

2 FOOTING SCHEDULE
 N.T.S.

- SHEET NOTES:**
- REFER TO S1 FOR STRUCTURAL GENERAL NOTES.
 - VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL AND BUILDING MANUFACTURER DRAWINGS. DO NOT SCALE DRAWINGS.
 - REFER TO S3.0 FOR DETAILS.
 - 8 INCH THICK CONCRETE SLAB-ON-GRADE OVER 6" OF COMPACTED CRUSHED ROCK REINFORCE SLAB WITH #5 BARS @ 18" O.C. BOTH WAYS AT SLAB MID-THICKNESS. USE 6 MIL. VAPOR BARRIER UNDER THE SLAB AT OWNER'S OPTION. MAX PERMISSIBLE LOAD ON SLAB IS 20,000 LBS.
 - 4 - #6 FRAME TIES PER 2 & 7/S3.0
 - #5 HAIR PINS AT END WALL POST PER 5/S3.0, TYP.
 - USE LIGHT WEIGHT EQUIPMENT TO COMPACT THE SOIL WITHIN 2 FEET AROUND FOUNDATION AND SLAB TURNDOWN/STEMWALL.
 - CENTER FOOTING UNDER POSTS AND WALLS UNLESS OTHERWISE NOTED ON PLANS AND/OR DETAILS
 - SLAB-ON-GRADE CONTROL JOINT PER 9/S3.0
 - EXTEND CONCRETE BEYOND OUT TO OUT STEEL DIMENSION SHOWN ON THE PLAN PER BUILDING MANUFACTURER, SOMETIMES IT IS 1 1/2" BUT MUST BE VERIFIED WITH BUILDING MANUFACTURER.

PSE Consulting Engineers, Inc.
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 836 Mason Way
 Medford OR, 97501
 Phone: (541) 858-8500
 Fax: (541) 778-4663
 infomd@structure1.com

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Project:
 80' x 125'
 Cable Building

999 Kelly Drive
 Bardstown, KY 40004

Owner / Client:
 DuroBeam
 Job # 152536



DATE	REVISION	SCHEDULE	DESCRIPTION

DRAWN BY: R.N.
 DS. BY: R.N.
 CHK BY: N.T.
 DATE: 06-03-2021

TITLE:
 FOUNDATION PLANS

PAGE NO:

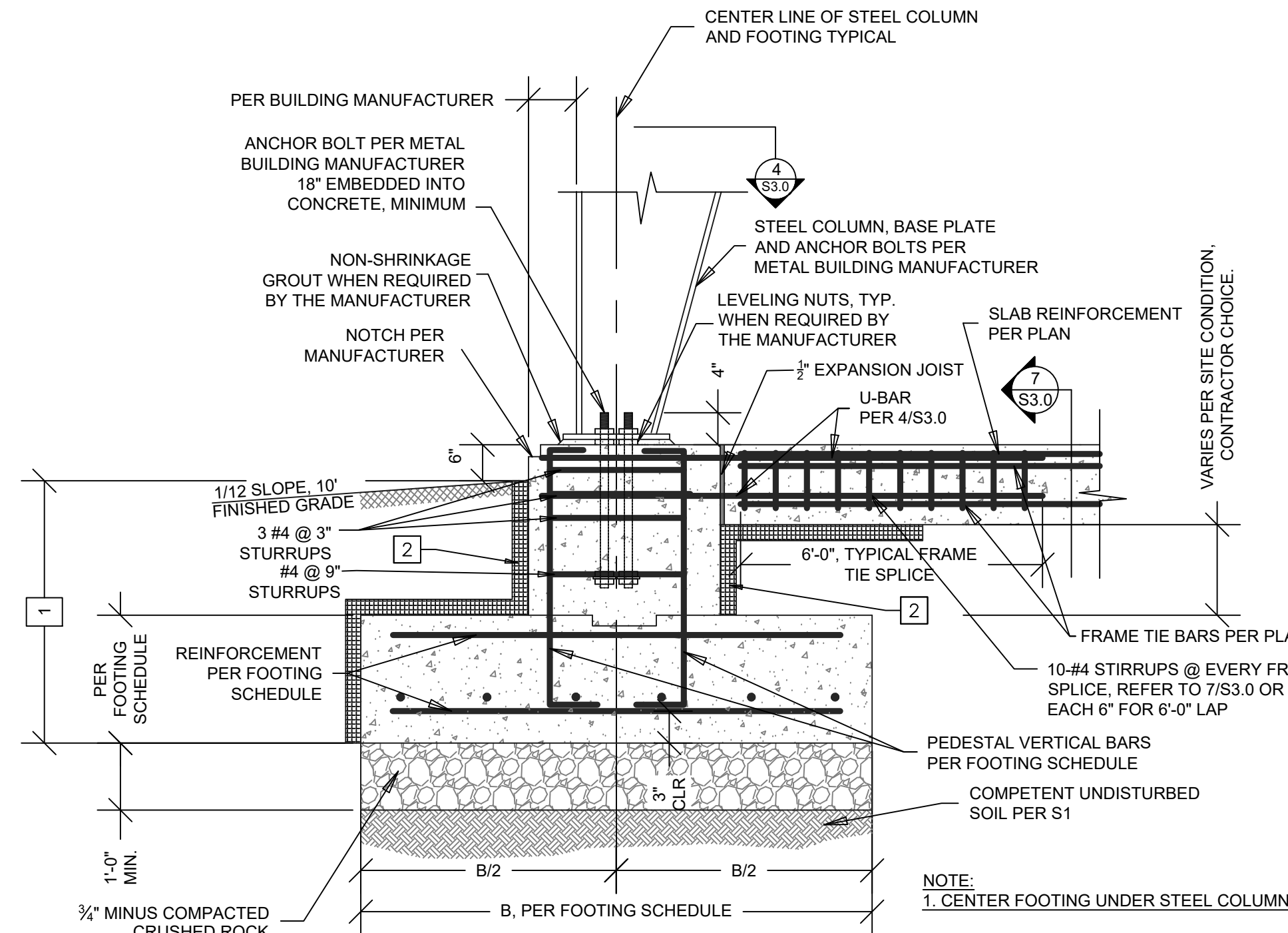
S2

PROJECT #:
 DUROBEAM 221-7

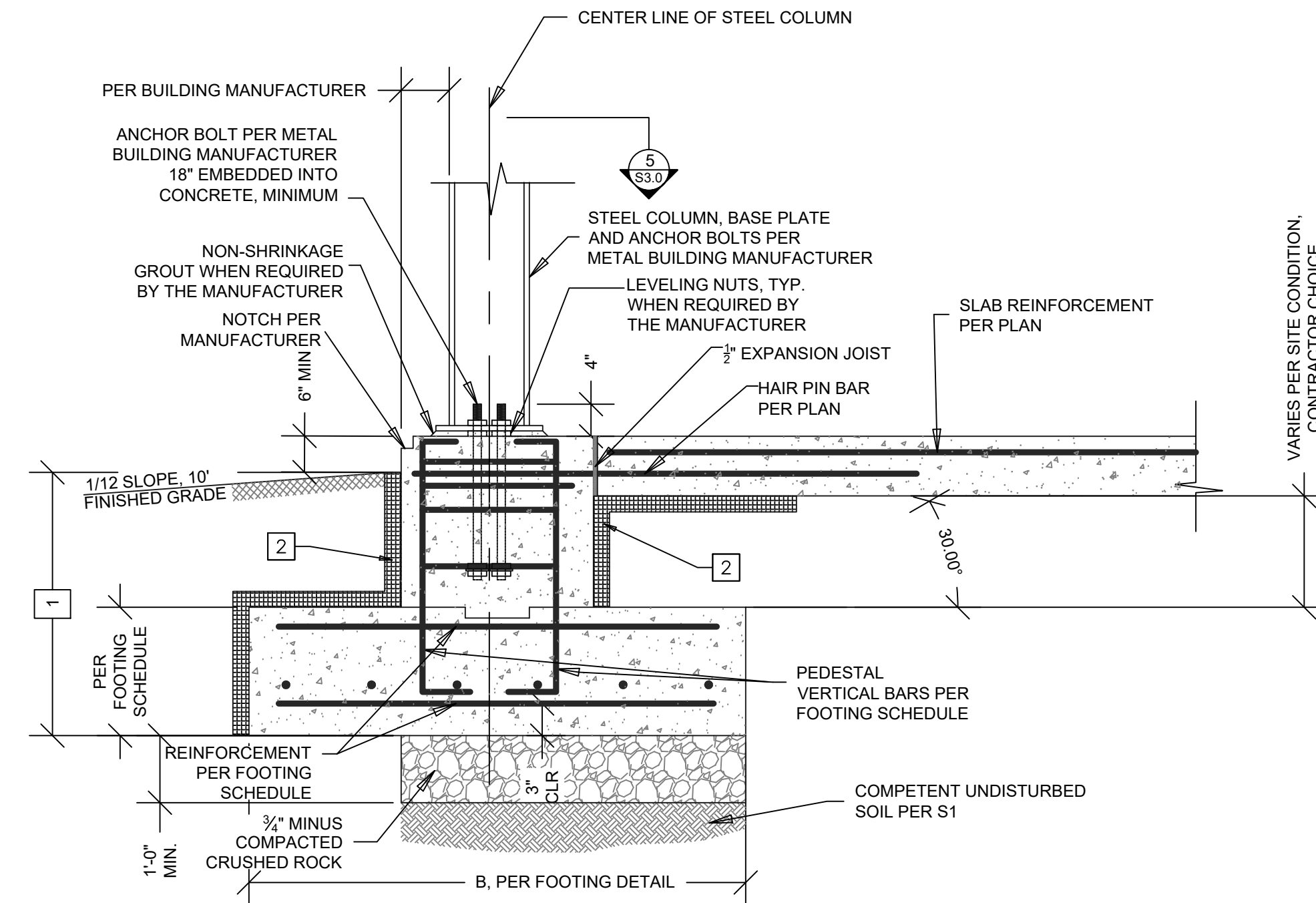
2021-06-03 Duro Beam 221-7 80x125 KY.dwg 6/3/2021 5:30 PM Riddhika Naik

ANCHOR BOLTS				
MARK	DIAMETER	# PER BP	EMBEDMENT (IN)	ANCHOR BOLT TYPE
F1	1"	4	1'-6"	HEAVY HEX BOLT F1554 Gr. 36
F2	5/8"	4	1'-6"	HEAVY HEX BOLT F1554 Gr. 36

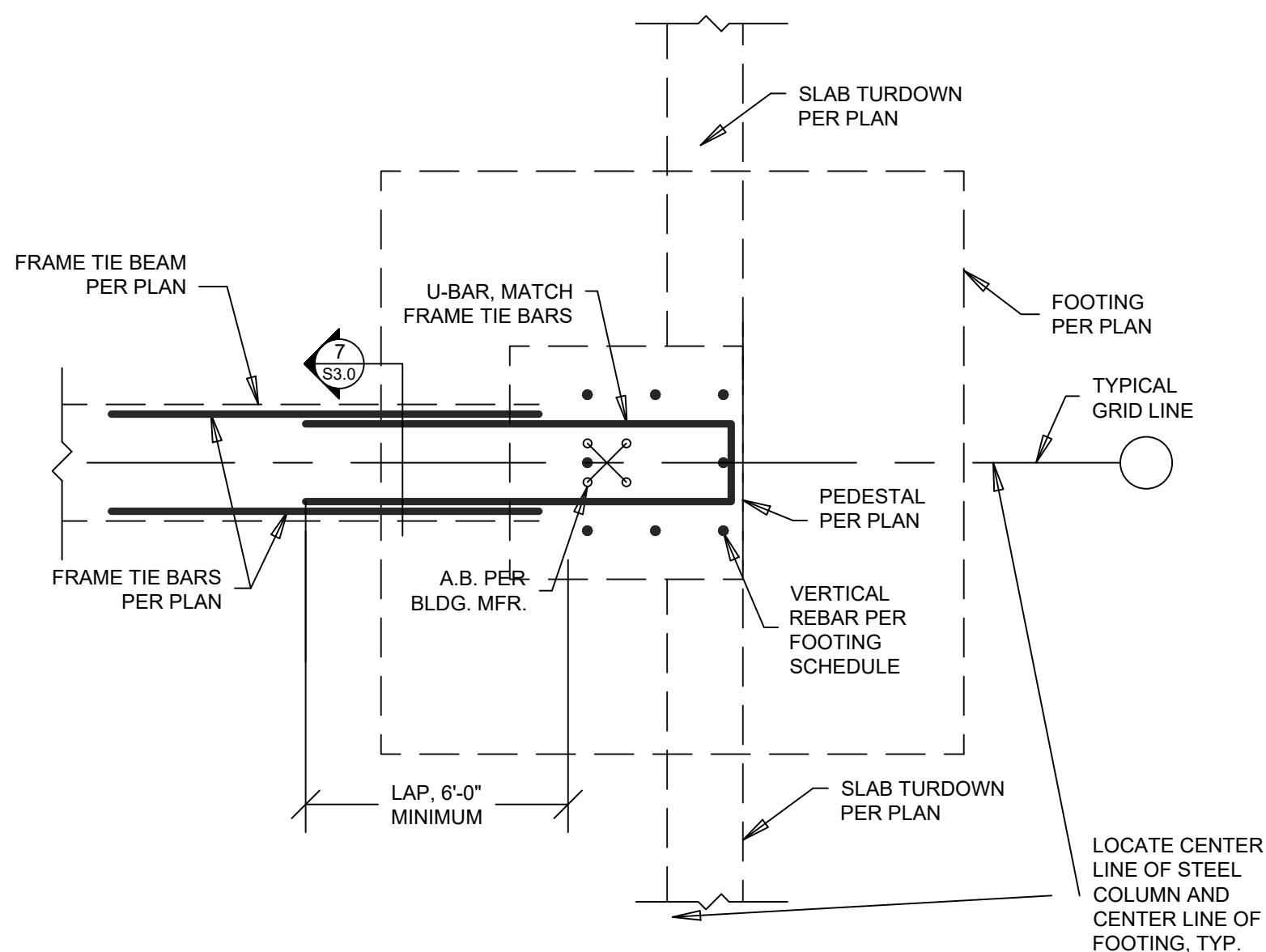
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N.T.S.



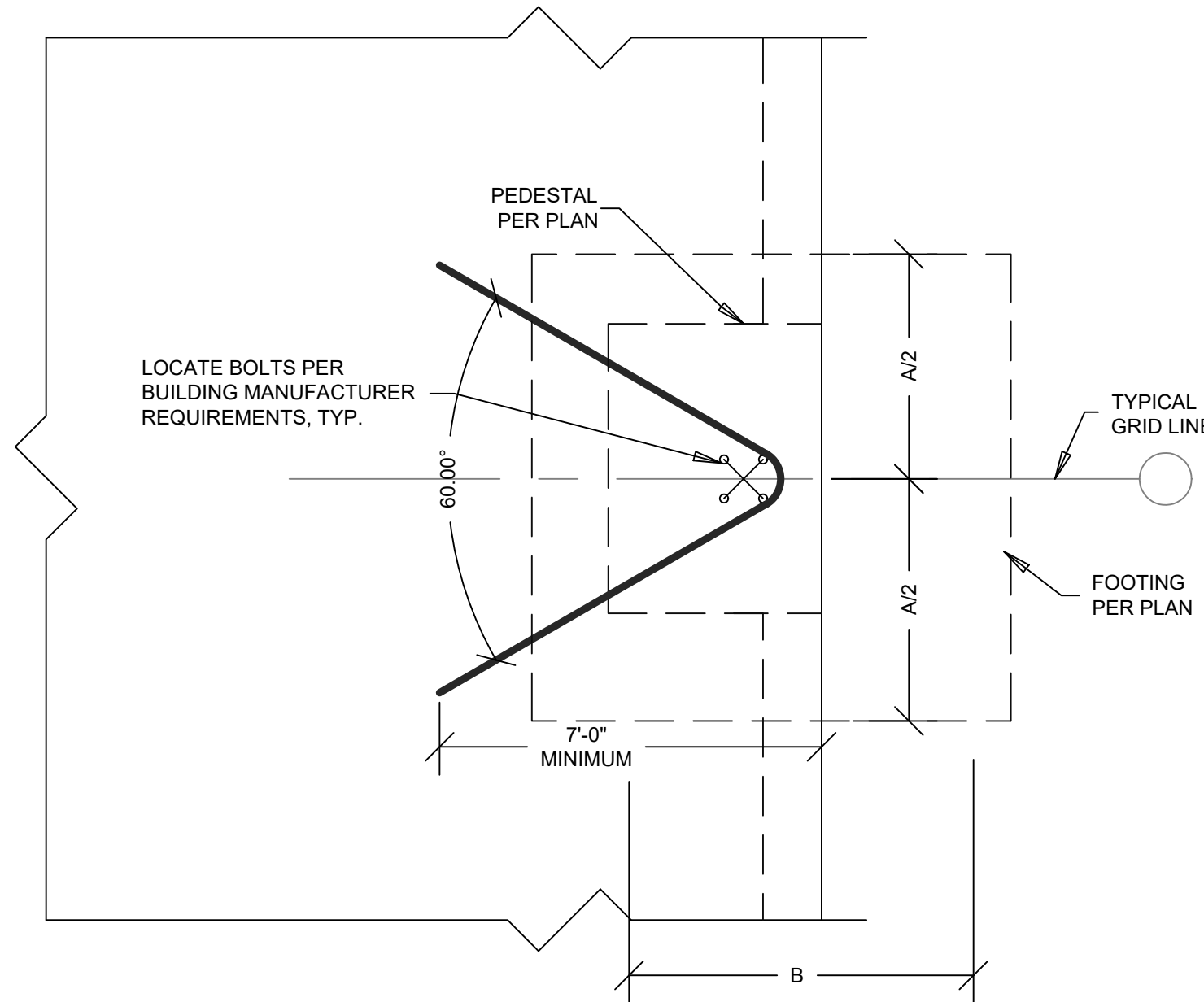
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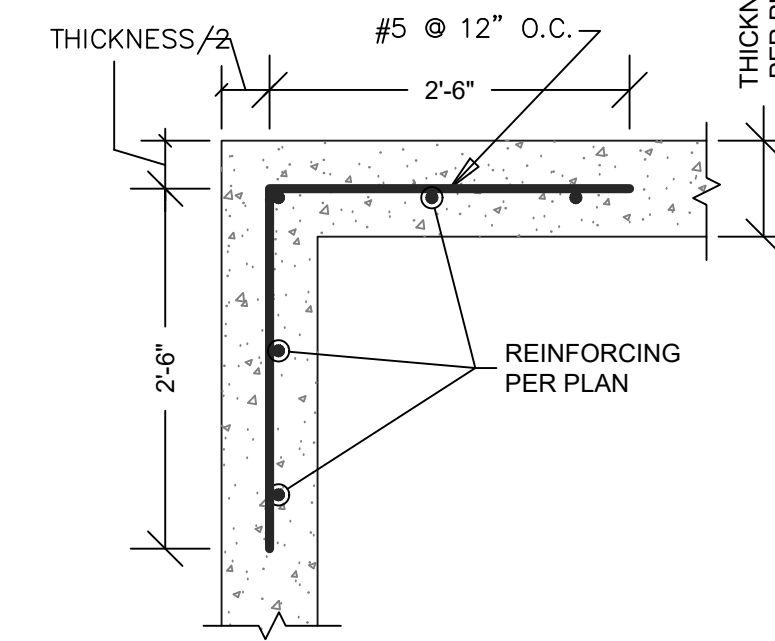
3 END WALL COLUMN FOOTING DETAIL
Scale: N.T.S.



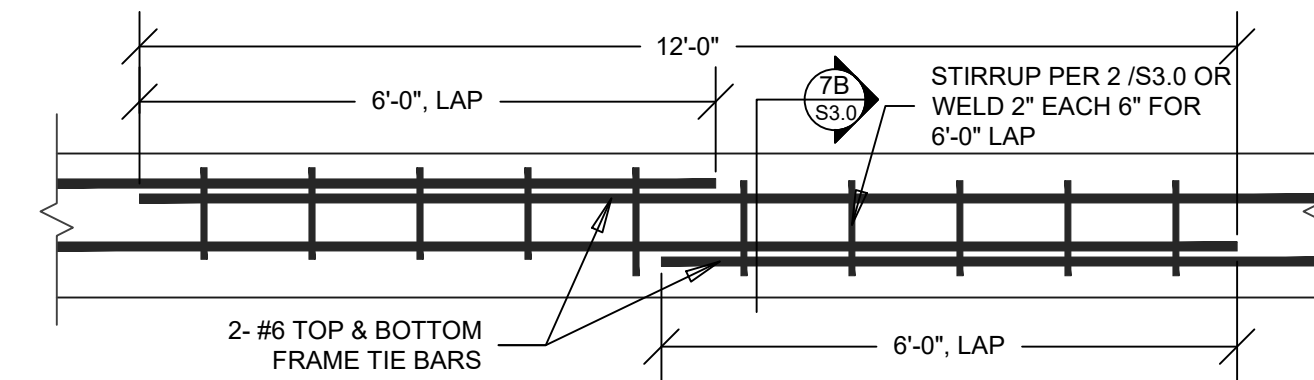
4 U BAR DETAILS
Scale: N.T.S.



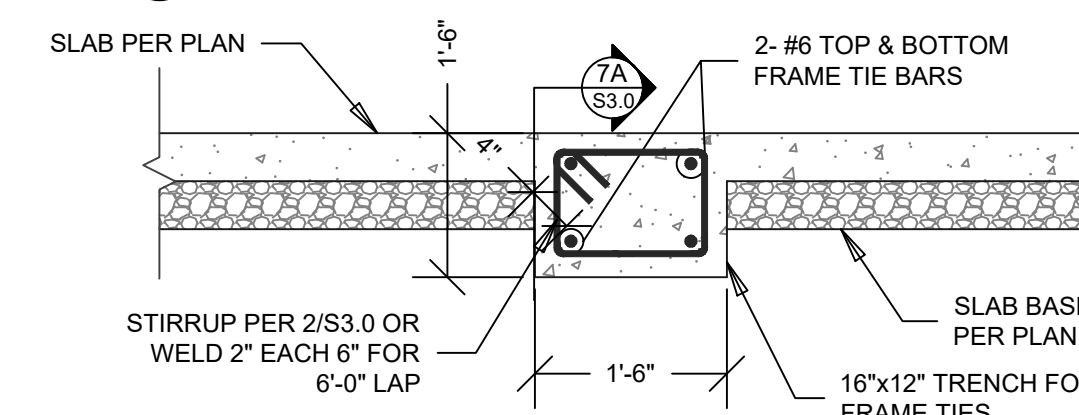
5 HAIR PIN BARS DETAILS
Scale: N.T.S.



6 CORNER REINFORCEMENT, TYPICAL
Scale: N.T.S.

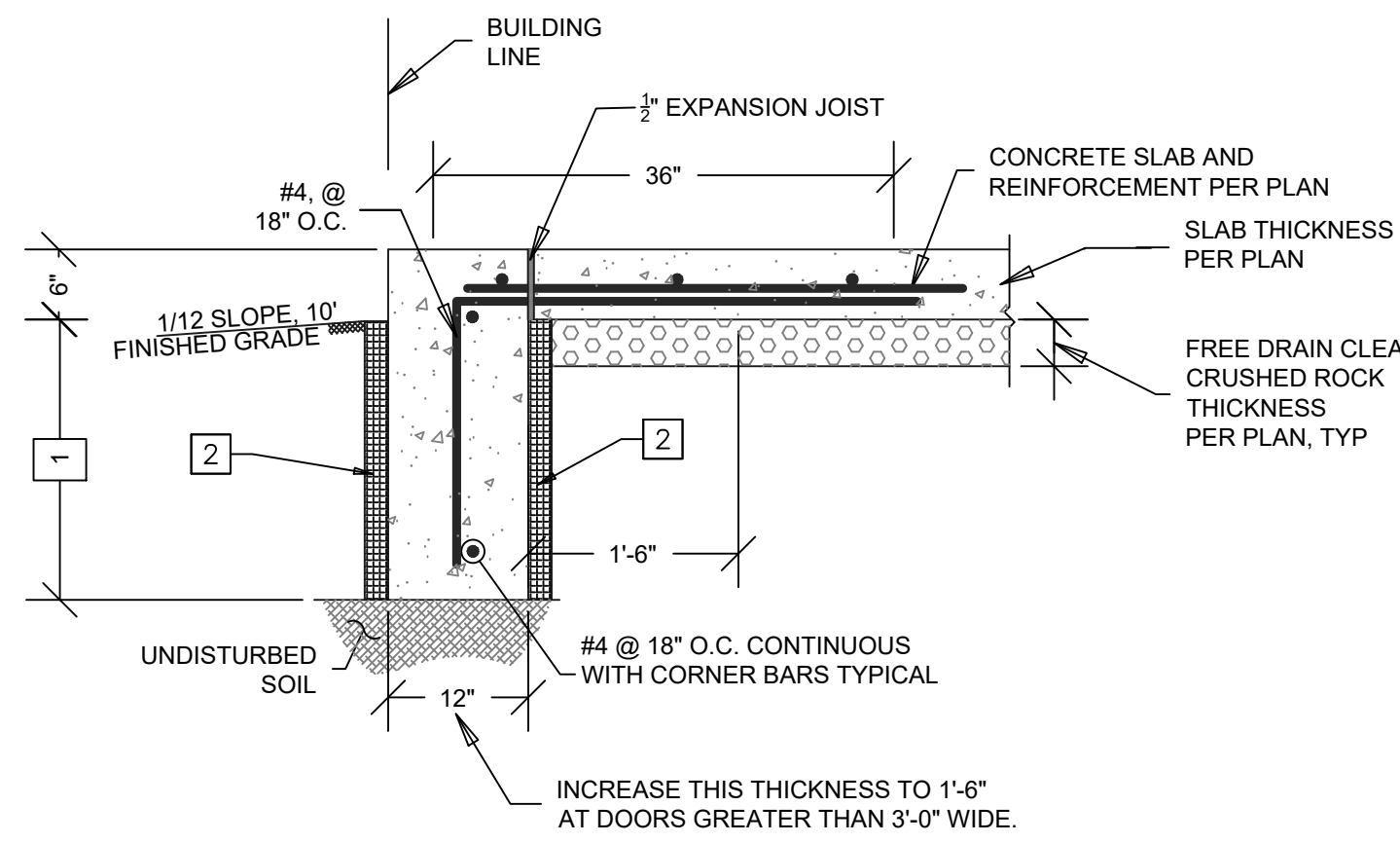


A FRAME TIE STAGGERED SPLICE DETAIL

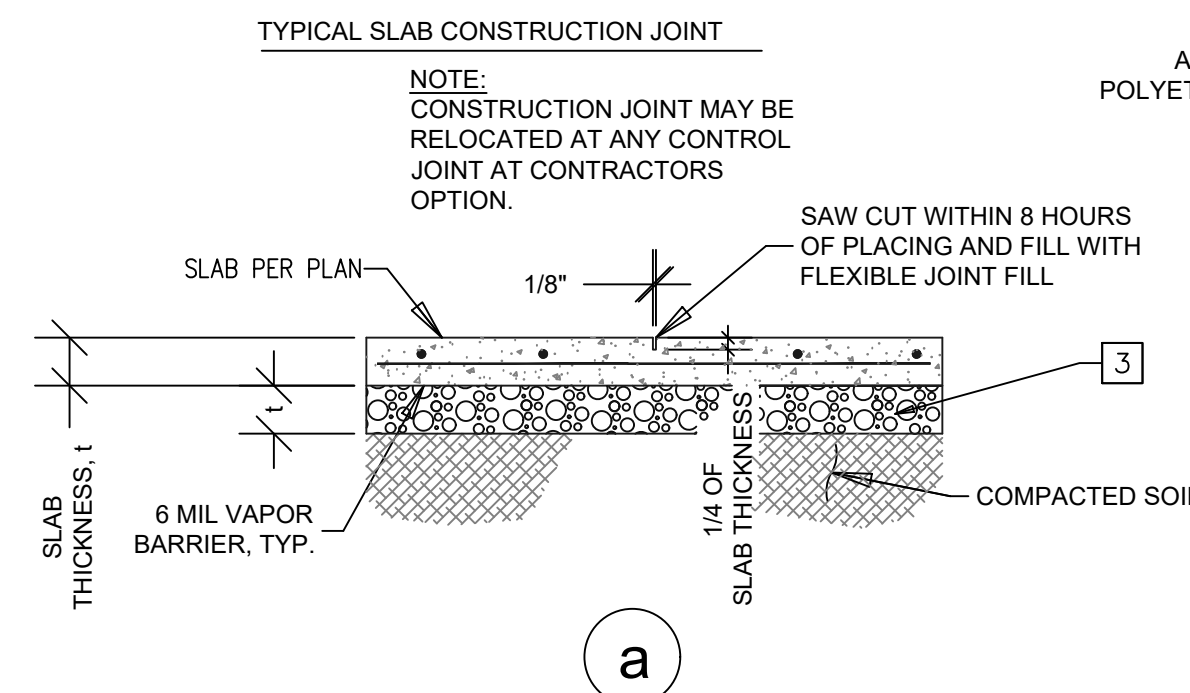


B FRAME TIE TRENCH SECTION

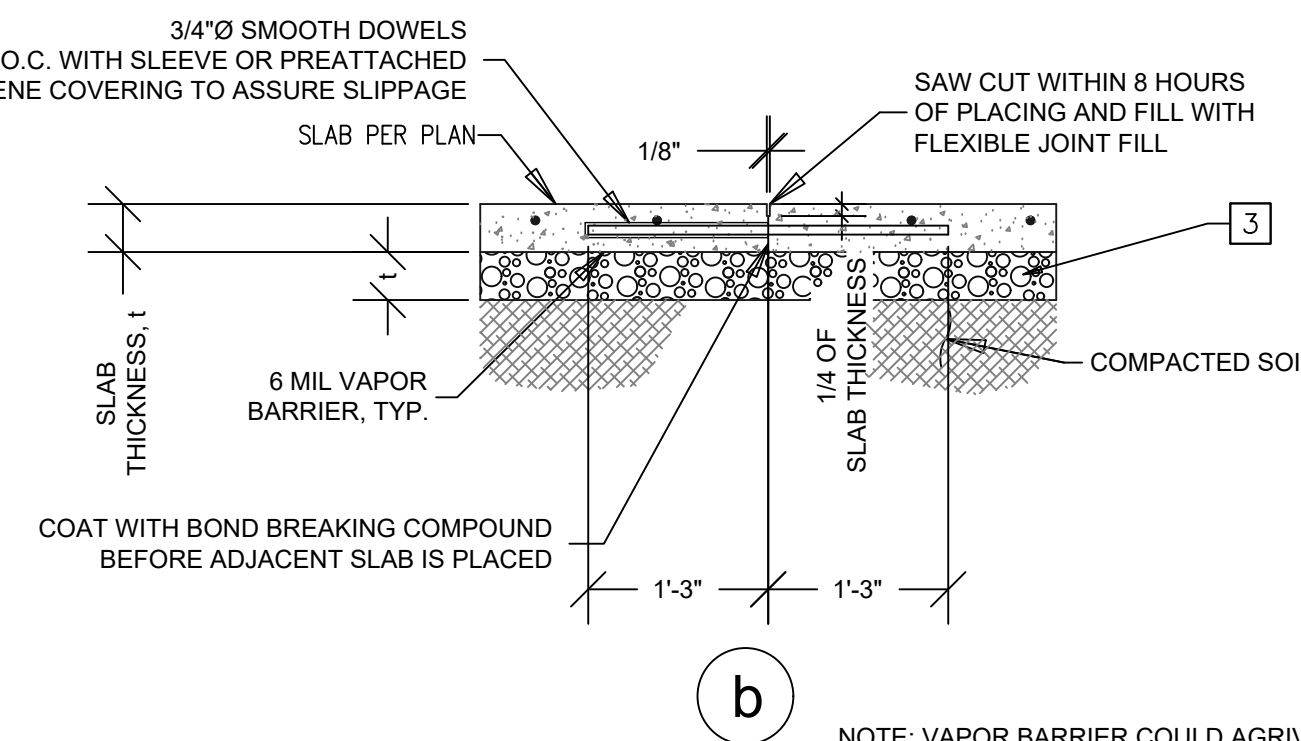
7 BUILDING FRAME TIE SECTIONS
Scale = N.T.S.



8 TYPICAL SLAB TURN DOWN
Scale: N.T.S.



9 TYPICAL SLAB CONSTRUCTION AND CONTROL JOINT
Scale: N.T.S.



NOTE: VAPOR BARRIER COULD AGGRAVATE SLAB CRACKING AND CURLING. VAPOR BARRIER MAYBE ELIMINATED. CONTRACTOR TO CHECK WITH OWNER AND THE BUILDING DEPARTMENT.

SHEET NOTES:

- 2'-6" OR FROST DEPTH PER LOCAL BUILDING DEPARTMENT, WHICHEVER IS GREATER CONTRACTOR TO VERIFY.
- MIN 2" R10 INSULATION IS REQUIRED AT HEATED AREA ONLY, CONSULT WITH YOUR ENERGY CONSULTANT.
- CONSULT WITH THE BUILDING ENERGY CONSULTANT IF RIGID INSULATION IS REQUIRED UNDER THE SLAB



PSE Consulting Engineers, Inc.

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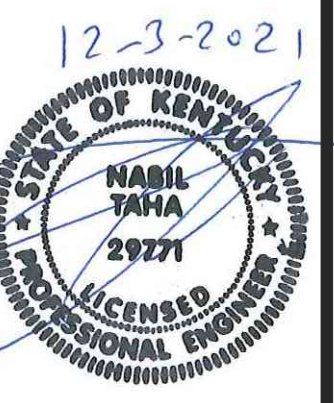
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Containers, and many more!
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And Green/Sustainable!

Project:
80' x 125'
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999 Kelly Drive
Bardstown, KY 40004

Owner / Client:
DuroBeam
Job # 152536



DATE	REVISION SCHEDULE	DESCRIPTION

DRAWN BY: R.N.
DS. BY: R.N.
CHK BY: N.T.
DATE: 06-03-2021

TITLE:
FOUNDATION
DETAILS
PAGE NO:

S3

PROJECT #:
DUROBEAM
221-7

